Devon Square Residential Wake Forest, NC

Prepared for:

Devon Square NC, LLC

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Traffic Impact Analysis
for
Devon Square Residential
Wake Forest, North Carolina

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Executive Summary

Kimley-Horn and Associates, Inc. has performed a Traffic Impact Analysis for the proposed Devon Square residential development located east of US 1 (Capital Boulevard) and south of Harris Road in Wake Forest, North Carolina. The site is currently occupied by a few single-family homes and as currently envisioned will consist of approximately 135 single family homes and 150 townhomes. The development is proposed to be accessed via one right-in/right-out driveway on US 1 and one full-movement driveway on Harris Road. Build-out of the project is anticipated in the year 2022.

This report presents trip generation, distribution, traffic analyses, and recommendations for transportation improvements required to meet anticipated traffic demands in conjunction with the development. The traffic conditions studied include the existing (2018) traffic condition as well as the projected (2022) background and build-out traffic conditions.

As shown in Table ES-1, the proposed development has the potential to generate 2,186 new trips during a typical weekday with 152 new trips during the AM peak hour and 201 new trips during the PM peak hour.

	Table ES-1 ITE Traffic Generation (Vehicles)								
Land Use	Land Use	Intensity Daily In Out		AM Peak Hour		PM Peak Hour			
Code				Out	ln	Out	ln	Out	
210	Single Family Detached	135	d.u.	685	685	25	76	86	50
221	Multifamily Housing (Townhomes)	150	d.u.	408	408	13	38	40	25
	Total Net New External Trips 1,093 1,093 38 114 126 75								

Capacity analyses were performed using Synchro Version 9.2 software. Table ES-2 summarizes the operation of the study intersections for the AM and PM peak hour traffic conditions.

Table ES-2 Level-of-Service Summary						
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)				
US 1 (Capital Boulevard) at Harris Road/Purnell Road (Signalized)						
Existing (2018) Traffic	C (27.7)	C (34.6)				
Background (2022) Traffic	D (37.6)	E (59.8)				
Build-out (2022) Traffic	D (40.4)	E (62.3)				



Table ES-2 (cont.) Level-of-Service Summary							
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)					
US 1 (Capital Boulevard) at Club Villas Drive (Unsignalized)							
Existing (2018) Traffic	EB – F (92.2) NBL – C (21.5) SBU – B (14.4)	EB – F (148.4) NBL – C (19.9) SBU – D (27.3)					
Background (2022) Traffic	EB – F (230.8) NBL – D (29.0) SBU – C (17.1)	EB – F (646.9) NBL – D (26.0) SBU – E (45.6)					
Build-out (2022) Traffic	EB – F (257.5) NBL – D (31.2) SBU – C (17.4)	EB – F (646.9) NBL – D (27.3) SBU – F (56.6)					
Harris Road at Wall	lridge Drive (Unsignalized	d)					
Existing (2018) Traffic	SB – B (12.1) EBL – A (7.8)	SB – B (13.5) EBL – A (8.0)					
Background (2022) Traffic	SB – B (13.4) EBL – A (7.9)	SB – C (17.2) EBL – A (8.2)					
Build-out (2022) Traffic	SB – B (14.2) EBL – A (8.0)	SB – C (18.1) EBL – A (8.3)					
Harris Road at Site	Driveway (Unsignalized)						
Build-out (2022) Traffic	NB – B (13.4) WBL – A (8.1)	NB – B (13.6) WBL – A (8.0)					
US 1 (Capital Boulevard)	at Site Driveway (Unsign	alized)					
Build-out (2022) Traffic	WB – C (16.7)	WB – D (34.3)					

The following roadway improvements were identified to be performed as part of this development to accommodate projected site traffic:

US 1 at Harris Road/Purnell Road:

• Extend the storage of the existing northbound left-turn lane on US 1 by approximately 100 feet to provide 400 feet of storage and appropriate tapers

US 1 at Site Driveway:

• Construct an exclusive northbound right-turn lane with 150 feet of storage on US 1

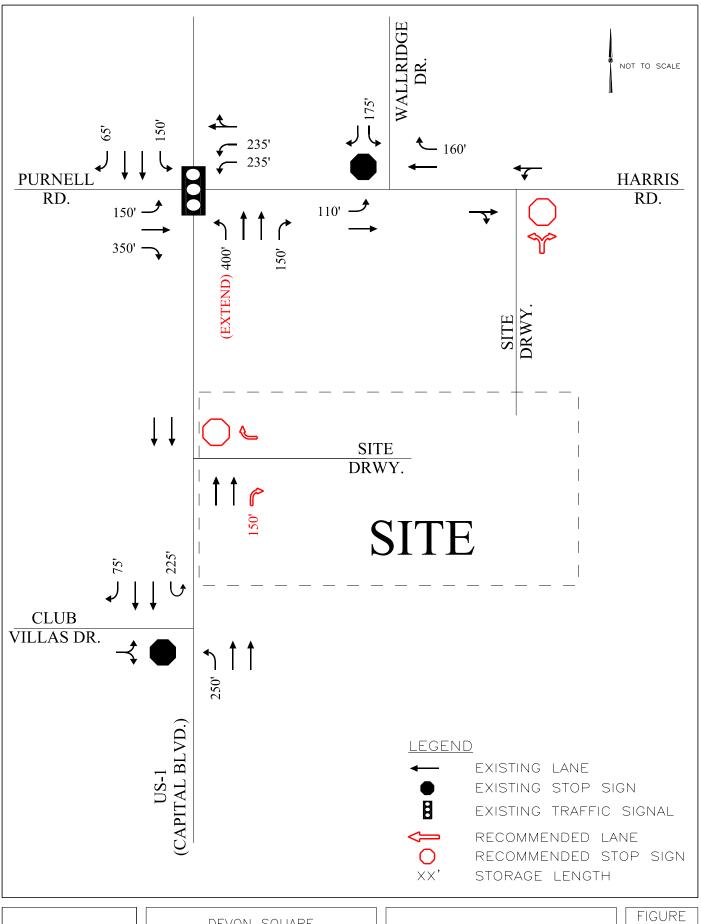
Analyses indicate that the intersections of US 1 at Club Villas Drive and US 1 at Harris Road/Purnell Road are expected to operate at an unacceptable LOS in the background and build-out conditions. All other intersections are expected to operate at acceptable levels-of-service at project build-out with the recommended improvements in place.

At the intersection of US 1 at Club Villas Drive, analyses indicate that the intersection is expected to operate with long delays on the minor street approach (Club Villas Drive) with or without the proposed development in place, and Synchro indicates that 95th percentile queues are expected to

increase by less than 10 feet with the addition of site traffic. Further, it is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. This intersection is not expected to meet Manual on Uniform Traffic Control Devices (MUTCD) traffic signal warrants.

Similarly, the intersection of US 1 at Harris Road at Purnell Road is expected to operate at LOS E in the PM peak hour with or without the proposed project in place. Project site traffic is expected to account for less than 3% of the total build-out traffic volume at that intersection. SimTraffic simulations indicate that all turning movement queues will be accommodated within the available storage with the possible exception of the northbound U-turn/left-turn in the AM peak hour. However, as SimTraffic does not accurately model U-turn/right-turn interactions, this is not expected to be an issue in reality. In addition, NCDOT project U-5307 proposes to convert US 1 to a freeway and convert this intersection to an interchange starting in the year 2024.

The recommended roadway laneage is shown on **Figure ES-1**.



DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

RECOMMENDED ROADWAY LANEAGE FIGURE ES-1



TABLE OF CONTENTS

			<u>Page No.</u>
1.0	INTE	RODUCTION	1
2.0	INVI	ENTORY	2
	2.1 2.2	STUDY AREAEXISTING CONDITIONS	
3.0	TRA	FFIC GENERATION	6
4.0	SITE	TRAFFIC DISTRIBUTION	7
5.0	PRO	JECTED TRAFFIC VOLUMES	9
	5.1 5.2 5.3 5.4 5.5	EXISTING TRAFFIC	9 9 10
6.0	CAP	ACITY ANALYSIS	15
	6.1 6.2 6.3 6.4 6.5	US 1 (CAPITAL BOULEVARD) AT HARRIS ROAD/PURNELL ROAD US 1 (CAPITAL BOULEVARD) AT CLUB VILLAS DRIVE HARRIS ROAD AT WALLRIDGE RIDGE HARRIS ROAD AT SITE DRIVEWAY US 1 (CAPITAL BOULEVARD) AT SITE DRIVEWAY	
7.0	REC	OMMENDATIONS	23
		Appendices	
Α.	Λοοι	JMPTIONS MEMORANDUM	
В.		GENERATION	
C.	TRAF	FIC COUNT DATA	
D.	APPF	ROVED DEVELOPMENT DATA	
E.	INTER	RSECTION SPREADSHEETS	
F.	SYNC	CHRO OUTPUT: EXISTING (2018)	
G.	SYNC	CHRO OUTPUT: BACKGROUND (2022)	
Н.	SYNC	CHRO OUTPUT: BUILD-OUT (2022)	
1.	SIGN	AL PLANS AND TIMING DATA	



LIST OF TABLES

Table No.	<u>Title</u>	Page No.
Table 3.0	ITE Traffic Generation (Vehicles)	6
Table 6.0-A	Level-of-Service Control Delay Thresholds	15
Table 6.0-B	Level-of-Service Summary	16
Table 6.1	Level-of-Service: US 1 (Capital Boulevard) at Harris Road/Purnell Road	18
Table 6.2	Level-of-Service: US 1 (Capital Boulevard) at Club Villas Drive	17
Table 6.3	Level-of-Service: Harris Road at Wallridge Drive	19
Table 6.4	Level-of-Service: Harris Road at Site Driveway	20
Table 6.5	Level-of-Service: US 1 (Capital Boulevard) at Site Driveway	21

LIST OF FIGURES

<u>Figure No.</u>	Title	Page No
Figure 1	Site Location	3
Figure 2	Preliminary Site Plan	4
Figure 3	Existing Roadway Laneage	5
Figure 4	Site Traffic Distribution and Percent Assignment	8
Figure 5	Existing and Projected (2022) Background AM Peak Hour Traffic Volume	s11
Figure 6	Existing and Projected (2022) Background PM Peak Hour Traffic Volume	s12
Figure 7	Projected (2022) Build-out AM Peak Hour Traffic Volumes	13
Figure 8	Projected (2022) Build-out PM Peak Hour Traffic Volumes	14
Figure 9	Recommended Roadway Laneage	23



1.0 Introduction

Kimley-Horn and Associates, Inc. has performed a Traffic Impact Analysis for the proposed Devon Square residential development located east of US 1 (Capital Boulevard) and south of Harris Road in Wake Forest, North Carolina. The site is currently occupied by a few single-family homes and as currently envisioned will consist of approximately 135 single family homes and 150 townhomes. The development is proposed to be accessed via one right-in/right-out driveway on US 1 and one full-movement driveway on Harris Road. Build-out of the project is anticipated in the year 2022.

This report presents trip generation, distribution, traffic analyses, and recommendations for transportation improvements required to meet anticipated traffic demands in conjunction with the development. The traffic conditions studied include the existing (2018) traffic condition and the projected (2022) background and build-out traffic conditions.

Town of Wake Forest and North Carolina Department of Transportation (NCDOT) transportation staff provided background data and were consulted regarding the elements to be covered in this analysis. The approved Memorandum of Understanding is included in the Appendix of this report.



2.0 Inventory

2.1 Study Area

The study area for this development in includes the following intersections:

- US 1 (Capital Boulevard) at Harris Road/Purnell Road
- US 1 (Capital Boulevard) at Club Villas Drive
- Harris Road at Wallridge Drive
- Harris Road at Site Driveway
- US 1 (Capital Boulevard) at Site Driveway

Figure 1 shows the site location. The preliminary site plan is shown on Figure 2.

2.2 Existing Conditions

The proposed Devon Square residential development is proposed to be located east of US 1 (Capital Boulevard) and south of Harris Road in Wake Forest, North Carolina. Roadways in the study area include US 1 (Capital Boulevard), Harris Road, Purnell Road, Wallridge Drive, and Club Villas Drive. The existing roadway laneage is shown in **Figure 3**.

US 1 (Capital Boulevard) is a 4-lane divided principal arterial with a posted speed limit of 55 mph in the vicinity of the site. The estimated 2018 average daily traffic (ADT) volume is approximately 31,000 vehicles per day (vpd) south of Harris Road. NCDOT has plans to upgrade US 1 to a freeway as part of STIP Project #U-5307, which will include limiting access along the corridor and constructing interchanges at several intersections, including at the Harris Road/Purnell Road intersection.

Harris Road is a 2-lane undivided major collector with a posted speed limit of 45 mph in the vicinity of the site. The estimated 2018 ADT volume is approximately 5,000 vpd east of Wallridge Drive. The Town of Wake Forest's 2010 Transportation Plan indicates that Harris Road is planned as a 2-lane divided section between Capital Boulevard and Oak Avenue.

Purnell Road is a 2-lane undivided major collector with a posted speed limit of 45 mph in the vicinity of US 1. The estimated 2018 ADT volume is approximately 4,500 vpd west of US 1. The Town of Wake Forest's 2010 Transportation Plan indicates that Purnell Road is planned as a 2-lane undivided roadway between Jackson Road and Capital Boulevard.

Wallridge Drive is a 2-lane undivided roadway with a posted speed limit of 25 mph. The estimated 2018 ADT volume is approximately 2,800 vpd at Harris Road.

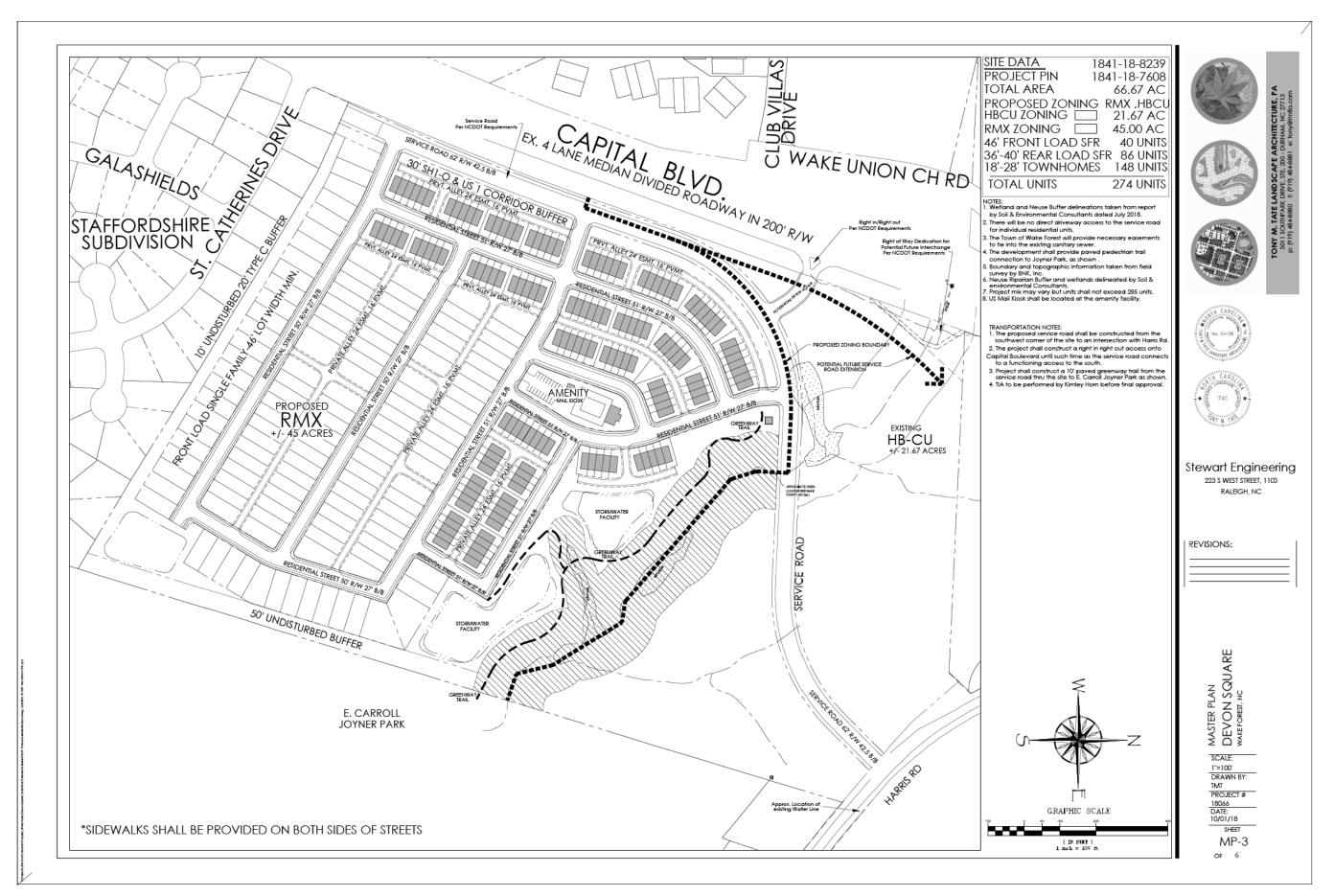
Club Villas Drive is a 2-lane undivided roadway with an assumed speed limit of 25 mph. The estimated 2018 ADT volume is less than 1,000 vpd.



DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

SITE LOCATION

FIGURE 1



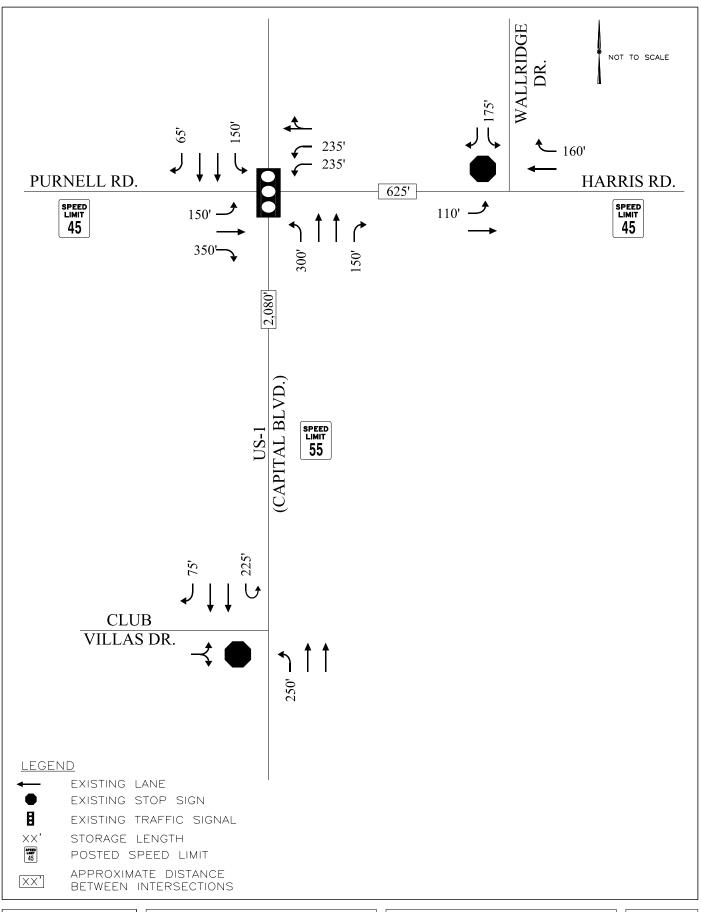
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DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

EXISTING ROADWAY LANEAGE

FIGURE

3



3.0 Traffic Generation

The traffic generation potential of the proposed development was determined using the traffic generation rates published in *Trip Generation* (Institute of Transportation Engineers, Tenth Edition, 2017). As currently envisioned the development will consist of approximately 135 single family homes and 150 townhomes. Table 3.0 summarizes the estimated traffic generation for the proposed development.

	Table 3.0 ITE Traffic Generation (Vehicles)								
Land Use	Land Use	Intensity Daily In Out		AM Peak Hour		PM Peak Hour			
Code				In	Out	ln	Out	ln	Out
210	Single Family Detached	135	d.u.	685	685	25	76	86	50
221	Multifamily Housing (Townhomes)	150	d.u.	408	408	13	38	40	25
	Total Net New External Trips 1,093 1,093 38 114 126 75								

Table 3.0 shows the proposed development has the potential to generate 2,186 new trips during a typical weekday with 152 new trips during the AM peak hour and 201 new trips during the PM peak hour.

Detailed trip generation calculations are included in the Appendix of this report.

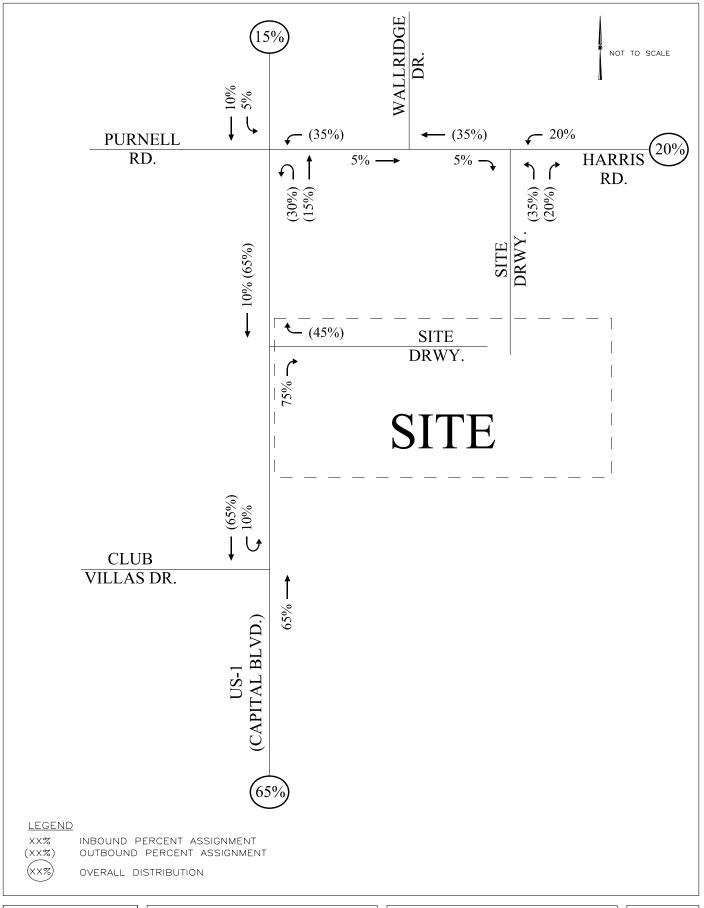


4.0 Site Traffic Distribution

The projected site-generated trips were assigned to the surrounding roadway network. The directional distribution and assignment for this development were based on a review of surrounding land uses and traffic patterns in the study area as well as a review of other traffic analyses performed in the area.

- 65% to/from the south on US 1
- 20% to/from the east on Harris Road
- 15% to/from the north on US 1

The site traffic distribution and percent assignment are shown on Figure 4.



DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

SITE TRAFFIC DISTRIBUTION AND PERCENT ASSIGNMENT

FIGURE

4



5.0 Projected Traffic Volumes

5.1 Existing Traffic

AM peak hour (6:30 to 9:00 AM) and PM peak hour (4:30 to 6:30 PM) turning movement counts were performed at the following intersections:

•	US 1 (Capital Boulevard) at Harris Road/Purnell Road	October 30, 2018
•	US 1 (Capital Boulevard) at Club Villas Drive	October 30, 2018
•	Harris Road at Wallridge Drive	October 30, 2018

The existing AM and PM peak hour traffic volumes are shown on **Figures 5** and **6**, and the traffic count data are included in the Appendix.

5.2 Historic Growth Traffic

Historic growth traffic is the increase in traffic due to usage increases and non-specific growth throughout the area. For this analysis, an annual growth rate of 3% was applied to the existing volumes up to the year 2022. Background growth calculations are detailed on intersection spreadsheets in the Appendix of this report.

5.3 Approved Development Traffic

Approved development traffic is generated by approved but not yet constructed projects in the vicinity of the proposed project. Based on discussions with the Town of Wake Forest, two projects were identified for inclusion in this analysis as background traffic: the Glen Oaks Residential development and the Planet Fitness in the Harris Crossing shopping center.

Per the *Glen Oaks TIA* (Ramey-Kemp, December 2017), at full build-out (projected in 2022) the Glenn Oaks Residential development will consist of approximately 225 single family homes and 73 townhomes east of US 1 across from Flex Way. All of the site traffic from that development was included in this analysis as background traffic. No geometric improvements were required of that development at any intersections analyzed as part of the Devon Square study.

A Planet Fitness is proposed to be constructed between US 1 and Wrigley Drive (adjacent to Richland Creek Elementary School) and north of the Harris Crossing shopping center. Access for this 27,718 SF fitness center will be provided through the Harris Crossing shopping center as well as to Wrigley Drive across from the Richland Creek Elementary School driveway. As no TIA was required of this project, site traffic was generated using the *ITE Trip Generation Manual* and assigned through the study area. It was assumed that this project would be in place by the year 2022.

Background traffic volumes consisting of existing, historic growth, and approved development traffic, are shown on **Figures 5** and **6** for the AM and PM peak hours, respectively.

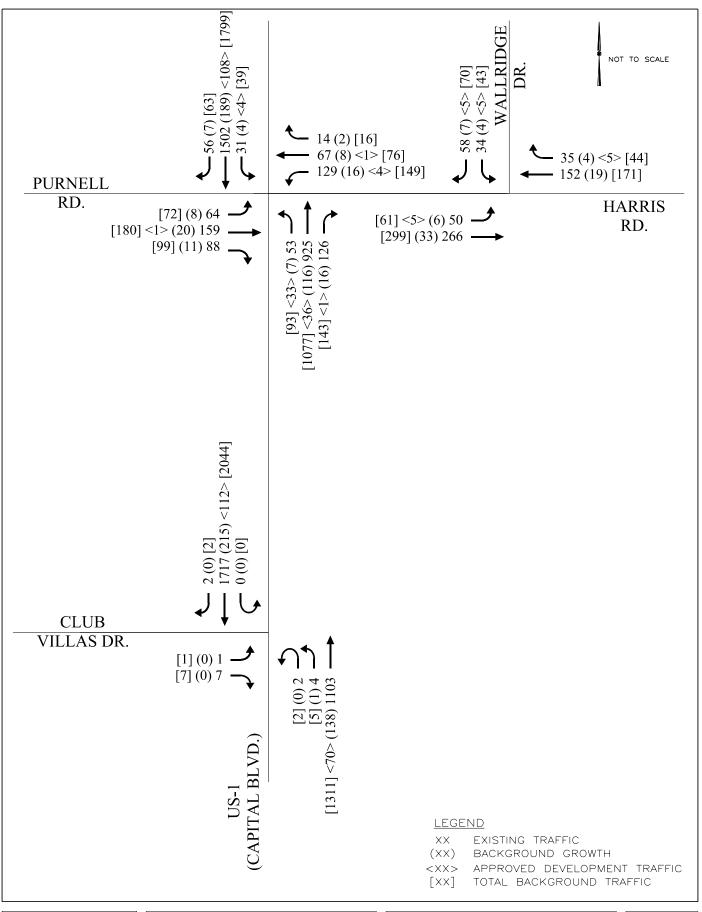


5.4 Site Traffic

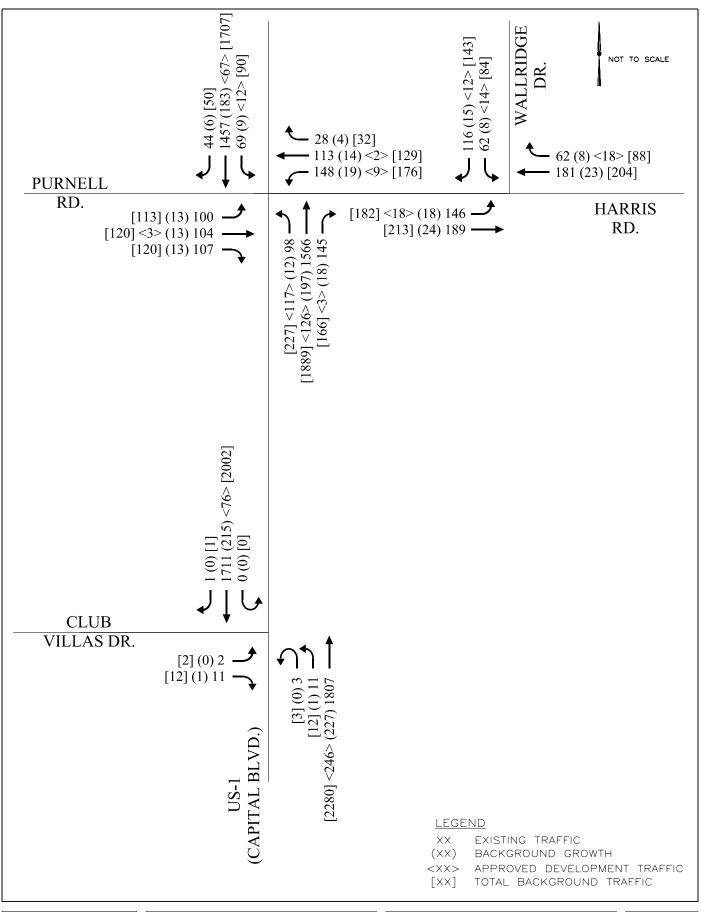
Project site traffic was generated and assigned to the adjacent roadway network according to the distribution discussed previously in Section 4.0. The site traffic volumes for the AM and PM peak hours are shown in **Figures 7** and **8**, respectively.

5.5 Build-Out Traffic

To obtain the projected (2022) build-out traffic volumes, site traffic was added to the projected (2022) background traffic. Traffic volume calculations are detailed in intersection spreadsheets in the Appendix of this report. **Figures 7** and **8** show the projected (2022) AM and PM peak hour build-out traffic volumes, respectively.

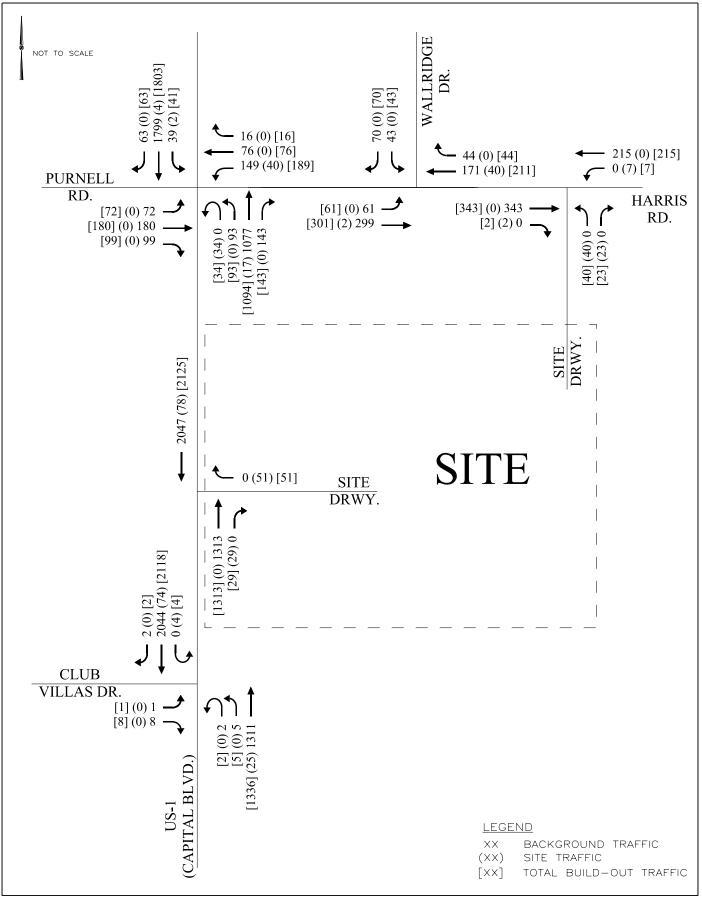


DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS EXISTING AND PROJECTED (2022) BACKGROUND AM PEAK HOUR TRAFFIC VOLUMES FIGURE 5



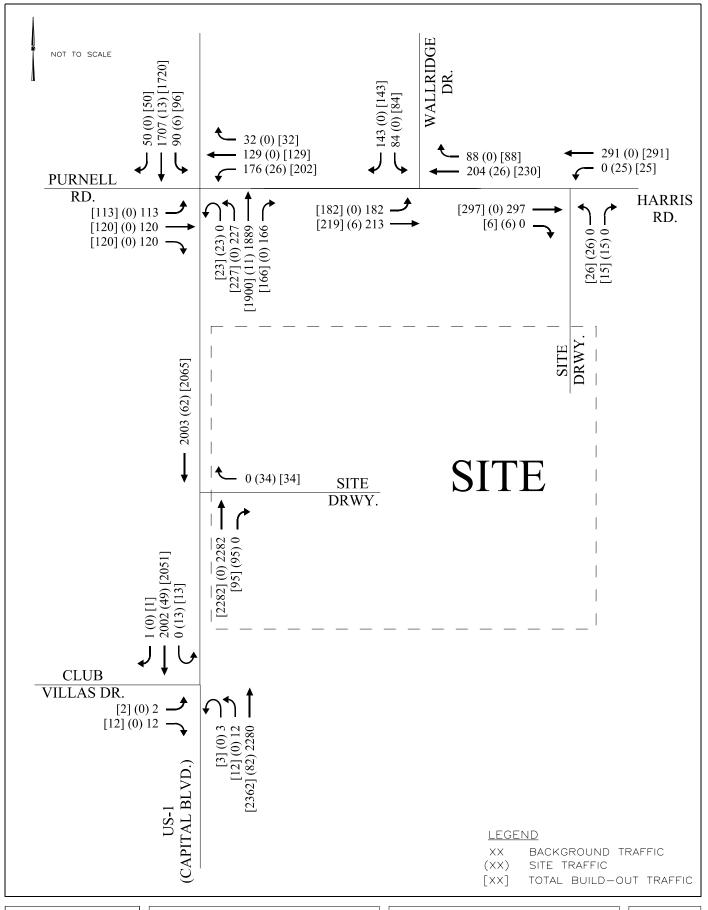
DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS EXISTING AND PROJECTED (2022) BACKGROUND PM PEAK HOUR TRAFFIC VOLUMES FIGURE

6



DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

PROJECTED (2022) BUILD-OUT AM PEAK HOUR TRAFFIC VOLUMES FIGURE 7



DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

PROJECTED (2022) BUILD-OUT PM PEAK HOUR TRAFFIC VOLUMES FIGURE

8



6.0 Capacity Analysis

Capacity analyses (see Appendix) were performed for the AM and PM peak hours for the existing traffic condition and the projected (2022) background and build-out traffic conditions using Synchro Version 9.2 software to determine the operating characteristics of the adjacent road network and the impacts of the proposed project.

Capacity is defined as the maximum number of vehicles that can pass over a particular road segment or through a particular intersection within a set time duration. Capacity is combined with Level-of-Service (LOS) to describe the operating characteristics of a road segment or intersection. LOS is a qualitative measure that describes operational conditions and motorist perceptions within a traffic stream. The *Highway Capacity Manual* defines six levels of service, LOS A through LOS F, with A representing the shortest average delays and F representing the longest average delays. LOS D is the typically accepted standard for signalized intersections in urbanized areas. For signalized intersections, LOS is defined for the overall intersection operation.

For unsignalized intersections, only the movements that must yield right-of-way experience control delay. Therefore, LOS criteria for the overall intersection is not reported by Synchro Version 9.2 or computable using methodology published in the *Highway Capacity Manual*. It is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. Table 6.0-A lists the LOS control delay thresholds published in the *Highway Capacity Manual* for signalized and unsignalized intersections.

	Table 6.0-A						
	Level-of-Service Control De	elay Thresholds					
Level-of- Service Signalized Intersections – Unsignalized Intersections – Average Control Delay [sec/veh]							
A	≤ 10	≤ 10					
В	> 10 – 20	> 10 – 15					
С	> 20 – 35	> 15 – 25					
D	> 35 – 55	> 25 – 35					
Е	> 55 - 80	> 35 – 50					
F	> 80	> 50					

Existing peak hour factors (PHF) were used at all existing intersections for all conditions except at new intersections, where a PHF of 0.90 was used. Existing signal timings, which were obtained during a field visit with NCDOT approval, were not adjusted as part of this analysis, and right-turns on red were allowed where currently allowed today.



Capacity analyses were performed for the existing (2018) traffic condition and the projected (2022) background and build-out traffic conditions for the following intersections:

- US 1 (Capital Boulevard) at Harris Road/Purnell Road
- US 1 (Capital Boulevard) at Club Villas Drive
- Harris Road at Wallridge Drive
- Harris Road at Site Driveway
- US 1 (Capital Boulevard) at Site Driveway

Table 6.0-B summarizes the LOS and delay (seconds per vehicle) for all of the study intersections for the existing (2018) traffic condition and the projected (2022) background and build-out traffic conditions. All capacity analyses are included in the Appendix and are briefly summarized in the following sub-sections.

Table 6.0-B Level-of-Service Summary					
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)			
US 1 (Capital Boulevard) at H	arris Road/Purnell Road	(Signalized)			
Existing (2018) Traffic	C (27.7)	C (34.6)			
Background (2022) Traffic	D (37.6)	E (59.8)			
Build-out (2022) Traffic	D (40.4)	E (62.3)			
US 1 (Capital Boulevard) a	t Club Villas Drive (Unsig	gnalized)			
Existing (2018) Traffic	EB – F (92.2) NBL – C (21.5) SBU – B (14.4)	EB – F (148.4) NBL – C (19.9) SBU – D (27.3)			
Background (2022) Traffic	EB – F (230.8) NBL – D (29.0) SBU – C (17.1)	EB – F (646.9) NBL – D (26.0) SBU – E (45.6)			
Build-out (2022) Traffic	EB – F (257.5) NBL – D (31.2) SBU – C (17.4)	EB – F (646.9) NBL – D (27.3) SBU – F (56.6)			
Harris Road at Wall	ridge Drive (Unsignalized	d)			
Existing (2018) Traffic	SB – B (12.1) EBL – A (7.8)	SB – B (13.5) EBL – A (8.0)			
Background (2022) Traffic	SB – B (13.4) EBL – A (7.9)	SB – C (17.2) EBL – A (8.2)			
Build-out (2022) Traffic	SB – B (14.2) EBL – A (8.0)	SB – C (18.1) EBL – A (8.3)			
Harris Road at Site	Driveway (Unsignalized)				
Build-out (2022) Traffic	NB – B (13.4) WBL – A (8.1)	NB – B (13.6) WBL – A (8.0)			



Table 6.0-B Level-of-Service Summary (cont.)						
Condition AM Peak Hour LOS (Delay) PM Peak Hour LOS (Delay)						
US 1 (Capital Boulevard) at Site Driveway (Unsignalized)						
Build-out (2022) Traffic WB – C (16.7) WB – D (34.3)						



6.1 US 1 (Capital Boulevard) at Harris Road/Purnell Road

Analyses indicate that the signalized intersection of US 1 (Capital Boulevard) at Harris Road/Purnell Road operates at LOS C in both the AM and PM peak hours. The intersection is expected to operate at LOS D in the AM peak hour and LOS E in the PM peak hour in the background traffic condition.

The following roadway improvements were identified to be performed as part of this development to accommodate projected site traffic:

• Extend the storage of the existing northbound left-turn lane on US 1 by approximately 100 feet to provide 400 feet of storage and appropriate tapers

At project build-out, the intersection is expected to continue to operate at LOS D in the AM peak hour and LOS E in the PM peak hour with only minor increases in overall intersection delay. Site traffic is expected to account for less than 4% of the total intersection volume at project build-out. SimTraffic simulations indicate that all turning movement queues will be accommodated within the available storage with the possible exception of the northbound U-turn/left-turn in the AM peak hour. However, as SimTraffic does not accurately model U-turn/right-turn interactions, this is not expected to be an issue in reality. In addition, NCDOT STIP Project U-5307 proposed to convert US 1 to a freeway section with an interchange at the intersection with Harris Road/Purnell Road starting in the year 2024. Therefore, no additional roadway improvements are recommended to accommodate projected site traffic.

Table 6.1 summarizes the operation of the intersection of US 1 (Capital Boulevard) at Harris Road/Purnell Road for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.1 Level-of-Service US 1 (Capital Boulevard) at Harris Road/Purnell Road (Signalized)						
Condition AM Peak Hour LOS (Delay) PM Peak Hour LOS (Delay)						
Existing (2018) Traffic	C (27.7)	C (34.6)				
Background (2022) Traffic	D (37.6)	E (59.8)				
Build-out (2022) Traffic	D (40.4)	E (62.3)				



6.2 US 1 (Capital Boulevard) at Club Villas Drive

Analyses indicate that the unsignalized intersection of US 1 (Capital Boulevard) at Club Villas Drive currently operates with long delays on the minor street approach (Club Villas Drive) in both the AM and PM peak hours. The intersection is expected to continue to operate with long delays in the year 2022 with or without the proposed project in place. Synchro indicates that 95th percentile queues are expected to increase by less than 10 feet with the addition of site traffic. It is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. This intersection is not expected to meet MUTCD traffic signal warrants.

No roadway improvements are recommended at this intersection.

Table 6.2 summarizes the operation of the intersection of US 1 (Capital Boulevard) at Club Villas Drive for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.2 Level-of-Service US 1 (Capital Boulevard) at Club Villas Drive (Unsignalized)				
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)		
Existing (2018) Traffic	EB – F (92.2) NBL – C (21.5) SBU – B (14.4)	EB – F (148.4) NBL – C (19.9) SBU – D (27.3)		
Background (2022) Traffic	EB – F (230.8) NBL – D (29.0) SBU – C (17.1)	EB – F (646.9) NBL – D (26.0) SBU – E (45.6)		
Build-out (2022) Traffic	EB – F (257.5) NBL – D (31.2) SBU – C (17.4)	EB – F (646.9) NBL – D (27.3) SBU – F (56.6)		



6.3 Harris Road at Wallridge Ridge

Analyses indicate that the unsignalized intersection of Harris Road at Wallridge Drive currently operates with short delays on the minor street approach (Wallridge Drive) in both the AM and PM peak hours. The intersection is expected to continue to operate with short delays in the year 2022 with or without the proposed project in place. No roadway improvements are recommended to be performed at this intersection.

Table 6.3 summarizes the operation of the intersection of Harris Road at Wallridge Drive for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.3 Level-of-Service Harris Road at Wallridge Drive (Unsignalized)			
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)	
Existing (2018) Traffic	SB – B (12.1) EBL – A (7.8)	SB – B (13.5) EBL – A (8.0)	
Background (2022) Traffic	SB – B (13.4) EBL – A (7.9)	SB – C (17.2) EBL – A (8.2)	
Build-out (2022) Traffic	SB – B (14.2) EBL – A (8.0)	SB – C (18.1) EBL – A (8.3)	



6.4 Harris Road at Site Driveway

A full-movement site driveway is proposed on Harris Road approximately 1,250 feet east of Wallridge Drive. Analyses indicate that the intersection will operate with short delays on the minor street approach (Site Driveway) at project build-out. No roadway improvements are recommended to be constructed as part of this development

Table 6.4 summarizes the operation of the intersection of Harris Road at the Site Driveway for the projected (2022) build-out traffic condition.

Table 6.4 Level-of-Service Harris Road at Site Driveway (Unsignalized)			
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)	
Build-out (2022) Traffic	NB – B (13.4) WBL – A (8.1)	NB – B (13.6) WBL – A (8.0)	



6.5 US 1 (Capital Boulevard) at Site Driveway

A right-in/right-out site driveway is proposed to be constructed on US 1 (Capital Boulevard) approximately 200 feet north of Club Villas Drive. The following roadway improvement is recommended to be constructed as part of this development:

• Construct an exclusive northbound right-turn lane with 150 feet of storage on US 1

Analyses indicate that with the recommended improvement in place the intersection will operate with short to moderate delays on the minor street approach (Site Driveway) at project build-out.

Table 6.5 summarizes the operation of the intersection of US 1 (Capital Boulevard) at the Site Driveway for the projected (2022) build-out traffic condition.

Table 6.5 Level-of-Service US 1 (Capital Boulevard) at Site Driveway (Unsignalized)			
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)	
Build-out (2022) Traffic	WB – C (16.7)	WB – D (34.3)	



7.0 Recommendations

The following roadway improvements were identified to be performed as part of this development to accommodate projected site traffic:

US 1 at Harris Road/Purnell Road:

• Extend the storage of the existing northbound left-turn lane on US 1 by approximately 100 feet to provide 400 feet of storage and appropriate tapers

US 1 at Site Driveway:

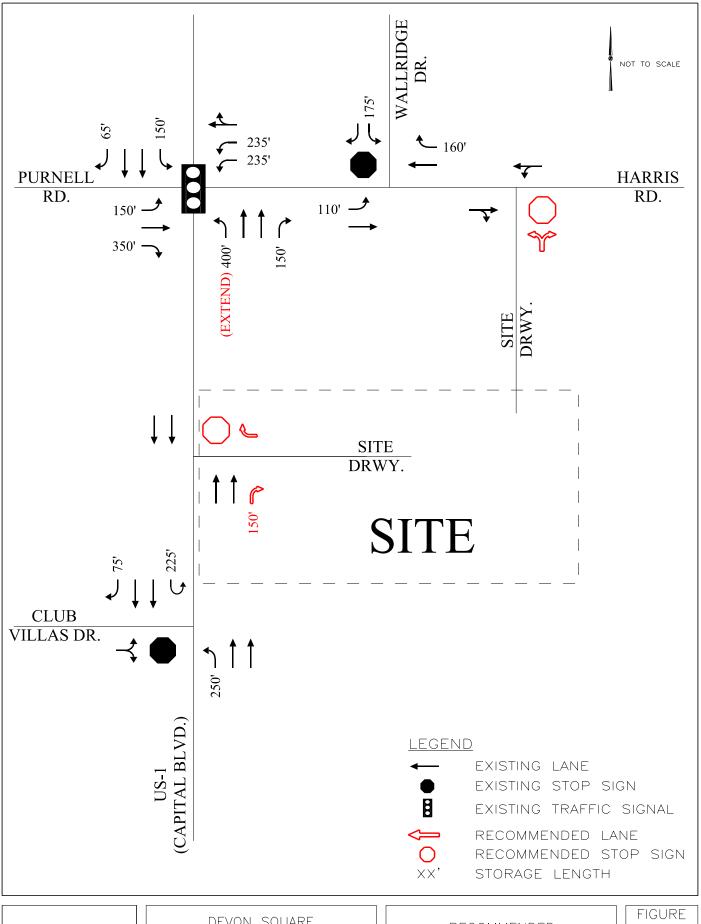
• Construct an exclusive northbound right-turn lane with 150 feet of storage on US 1

Analyses indicate that the intersections of US 1 at Club Villas Drive and US 1 at Harris Road/Purnell Road are expected to operate at an unacceptable LOS in the background and build-out conditions. All other intersections are expected to operate at acceptable levels-of-service at project build-out with the recommended improvements in place.

At the intersection of US 1 at Club Villas Drive, analyses indicate that the intersection is expected to operate with long delays on the minor street approach (Club Villas Drive) with or without the proposed development in place, and Synchro indicates that 95th percentile queues are expected to increase by less than 10 feet with the addition of site traffic. Further, it is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. This intersection is not expected to meet Manual on Uniform Traffic Control Devices (MUTCD) traffic signal warrants.

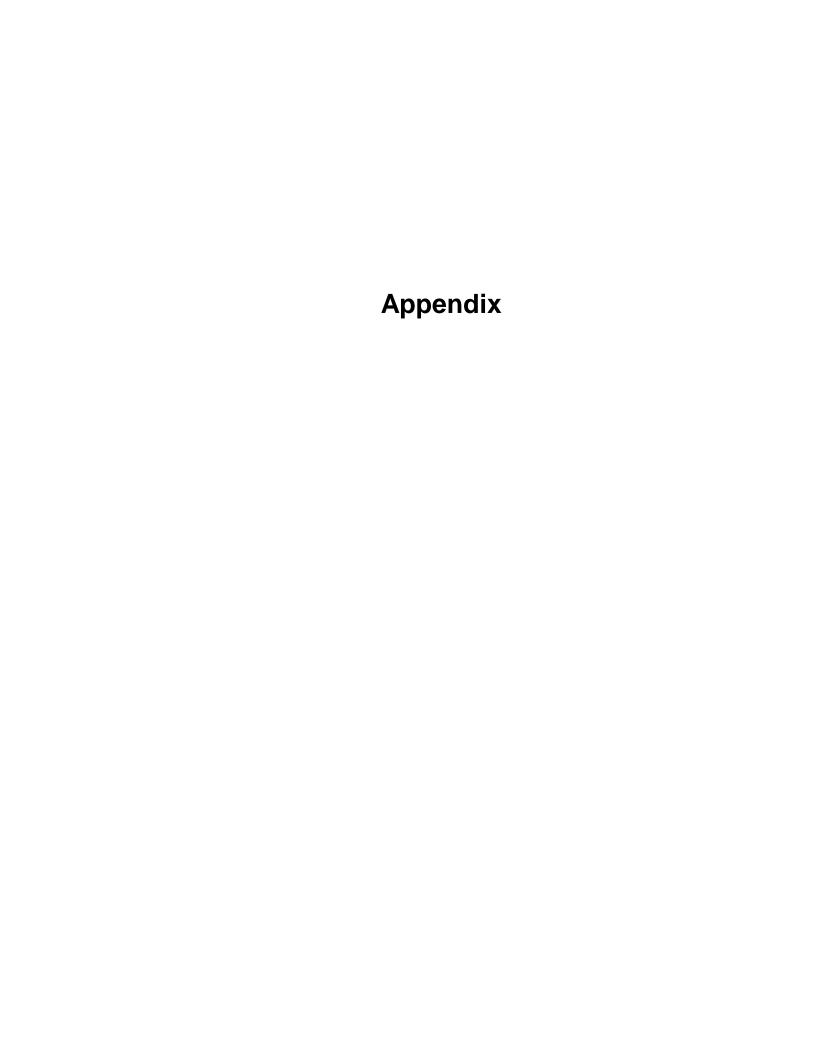
Similarly, the intersection of US 1 at Harris Road at Purnell Road is expected to operate at LOS E in the PM peak hour with or without the proposed project in place. Project site traffic is expected to account for less than 3% of the total build-out traffic volume at that intersection. SimTraffic simulations indicate that all turning movement queues will be accommodated within the available storage with the possible exception of the northbound U-turn/left-turn in the AM peak hour. However, as SimTraffic does not accurately model U-turn/right-turn interactions, this is not expected to be an issue in reality. In addition, NCDOT project U-5307 proposes to convert US 1 to a freeway and convert this intersection to an interchange starting in the year 2024.

The recommended roadway laneage is shown on **Figure 9**.



DEVON SQUARE WAKE FOREST, NC TRAFFIC CAPACITY ANALYSIS

RECOMMENDED ROADWAY LANEAGE FIGURE 9



Appendix A: Assumptions Memorandum

Preliminary Assumptions Devon Square Residential - Traffic Impact Analysis Wake Forest, North Carolina

KHA will perform analyses for the residential portion of the proposed Devon Square development, located south of Harris Road and east of US 1 in Wake, North Carolina. The following assumptions will be used in the analysis of the site:

The study area will consist of the following intersections:

- US 1 (Capital Boulevard) at Harris Road/Purnell Road
- US 1 (Capital Boulevard) at Club Villas Drive
- Harris Road at Wallridge Drive
- Harris Road at Site Driveway
- US 1 (Capital Boulevard) at Site Driveway

Based on other traffic counts performed in this part of Wake Forest, traffic counts will be performed at the existing intersections for the hours of 6:30-8:30 AM and 4:30-6:30 PM in order to capture the peak commuter traffic.

The study scenarios will consist of:

- Existing (2018)
- Background (2022)
- Build-out (2022)

No analyses will be performed with Capital Boulevard converted to a freeway since the freeway conversion project (NCDOT Project U-5307) is not expected to reach this area until the year 2024.

Based on discussions with the Town of Wake Forest and the North Carolina Department of Transportation (NCDOT), there are two approved developments in the study area that were identified for inclusion as background traffic: the proposed Planet Fitness project and the Glen Oaks Residential development. In addition to the approved development traffic, an annual growth rate of 3% will also be applied to the existing traffic volumes up to the year 2022.

The following directional distribution will be used for the site based on a review of surrounding land uses (see attached distribution figure):

- 65% to/from the south on US 1
- 20% to/from the east on Harris Road
- 15% to/from the north on US 1

The property is currently vacant, and as currently envisioned the residential portion of the development will consist of approximately 126 single family dwelling units and 149 townhomes, which are anticipated to be 3 stories. Trips will be generated using ITE Trip Generation 10th Edition rates. See attached trip generation table.

No analysis will be performed for the commercial portion of the property at this time. A separate TIA will be required for that development at the time it moves forward.



Job	Subject	Job No.	

Expect More. Experience Better.

Designed By ______ Date _____ Date _____

Sheet No. ____ of ___

Devon Square -	Distriby	700		
	(15%)			
	5			
Purnell RS		Harris Rd	20%	
				•
		Site		
	(65%)			
Or	ne of FORTUNE's 100	Best Companies to Worl	(For	

Devon Square Table 1 - Trip Generation Daily **AM Peak Hour PM Peak Hour Land Use** Land Use Intensity Code Total Total In Total Out In Out In Out Single Family Detached Housing 126 d.u. 1,286 643 643 94 24 70 127 80 47 Multifamily Housing (Mid-Rise) 810 149 d.u. 405 405 51 13 38 65 40 25

2,096

1,048

1,048

145

37

108

192

120

K:\RAL_TPTO_Traffic\018772001 Devon Square\T4 - Analysis\[018772001 Devon Square TIA Data.xls]Trip Gen

Total Net New External Trips

210

221

10/30/18

72

Appendix B: Trip Generation

Devon Square Table 1 - Trip Generation Daily **AM Peak Hour PM Peak Hour** Intensity Total Total In Out Total In Out In Out 1,370 25 135 d.u. 685 685 101 76 136 86 50

51

152

13

38

38

114

65

201

40

126

408

1,093

408

1,093

816

2,186

K:\RAL_TPTO_Traffic\018772001 Devon Square\T4 - Analysis\[018772001 Devon Square TIA Data.xls]Trip Gen

150

d.u.

Land Use

Single Family Detached Housing

Multifamily Housing (Mid-Rise)

Total Net New External Trips

Land Use

Code

210

221

25

75

Appendix C: Traffic Count Data

US 1 and Purnell Road/Harris Road AM and PM Peak Hour Traffic Count Count Performed: Tuesday, October 30, 2018

		US 1		Н	arris Ro	ad		US 1		Pι	ırnell Ro	ad	Intersection
	S	outhbou	nd	V	/estbou	nd	N	orthbou	nd	E	astbour	nd	Volume
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
6:30	5	422	6	34	3	3	3	161	13	8	23	21	702
6:45	11	511	16	32	13	2	11	203	36	16	65	24	940
7:00	5	322	13	47	9	2	9	213	54	23	59	27	783
7:15	7	349	16	29	20	7	17	242	21	9	19	21	757
7:30	8	320	11	21	25	3	16	267	15	16	16	16	734
7:45	5	371	16	36	18	4	25	251	14	12	24	21	797
8:00	10	353	17	32	23	6	23	250	25	8	12	26	785
8:15	10	316	10	45	16	1	16	234	36	15	23	23	745
16:30	14	398	16	33	23	4	23	389	42	17	19	15	993
16:45	18	359	8	38	25	9	17	391	35	23	36	22	981
17:00	17	367	9	40	27	8	23	388	35	27	24	22	987
17:15	18	379	15	38	35	6	26	358	39	28	30	30	1,002
17:30	16	352	12	32	26	5	32	429	36	22	14	33	1,009
17:45	12	297	13	39	28	13	21	401	57	29	37	33	980
18:00	12	342	10	28	26	3	29	396	47	21	19	23	956
18:15	10	261	11	38	21	8	26	343	29	25	29	24	825
	-												-
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
6:30 - 7:30	28	1,604	51	142	45	14	40	819	124	56	166	93	3,182
6:45 - 7:45	31	1,502	56	129	67	14	53	925	126	64	159	88	3,214
7:00 - 8:00	25	1,362	56	133	72	16	67	973	104	60	118	85	3,071
7:15 - 8:15	30	1,393	60	118	86	20	81	1,010	75	45	71	84	3,073
7:30 - 8:30	33	1,360	54	134	82	14	80	1,002	90	51	75	86	3,061
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
16:30 - 17:30	67	1,503	48	149	110	27	89	1,526	151	95	109	89	3,963
16:45 17:45	60	1 /57	11	1/10	112	28	08	1 566	1/15	100	104	107	2 070

16:30 - 17:30	67	1,503	48	149	110	27	89	1,526	151	95	109	89	3,963
16:45 - 17:45	69	1,457	44	148	113	28	98	1,566	145	100	104	107	3,979
17:00 - 18:00	63	1,395	49	149	116	32	102	1,576	167	106	105	118	3,978
17:15 - 18:15	58	1,370	50	137	115	27	108	1,584	179	100	100	119	3,947
17:30 - 18:30	50	1,252	46	137	101	29	108	1,569	169	97	99	113	3,770

					Peak-H	lour Tra	ffic Volu	ımes					
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Volume
6:45 - 7:45	53	925	126	31	1,502	56	64	159	88	129	67	14	3,214
16:45 - 17:45	98	1,566	145	69	1,457	44	100	104	107	148	113	28	3,979

				F	eak-Ho	ur Facto	r by Mo	vement					
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	PHF
6:45 - 7:45	0.779	0.866	0.583	0.705	0.735	0.875	0.696	0.612	0.815	0.686	0.670	0.500	0.855
16:45 - 17:45	0.766	0.913	0.929	0.958	0.961	0.733	0.792	0.722	0.811	0.925	0.807	0.778	0.986

		Peak-Hour Facto	or by Approach											
Peak Hour														
6:45 - 7:45	0.93	0.74	0.71	0.91	0.86									
16:45 - 17:45	0.91	0.95	0.88	0.92	0.99									

	Heavy Vehicle Percentage by Movement												
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	%HV
6:45 - 7:45	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
16:45 - 17:4	5 0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

		Heavy Vehicle Perce	ntage by Approach									
Peak HourNBSBEBWB%HV												
6:45 - 7:45	0%	0%	0%	0%	0%							
16:45 - 17:45	0%	0%	0%	0%	0%							

US 1 and Club Villas Drive/ AM and PM Peak Hour Traffic Count Count Performed: Tuesday, October 30, 2018

j 										<u> </u>			
		US 1			0			US 1			Villas I		Intersection
0, ,		uthbou			estbour			orthbou			astbour		Volume
Start Time	U-Turn	Thru	Right	Left	Thru	Right	U-Turn	Left	Thru	Left	Thru	Right	4
6:30	0		0	0	0	0	2	0		0	0	2	4
6:45	0		0	0	0	0	0	0		0	0	3	3
7:00	0		0	0	0	0	1	0		0	0	3	4
7:15	0		1	0	0	0	0	0		0	0	1	2
7:30	0		0	0	0	0	0	1		0	0	0	1
7:45	0		1	0	0	0	1	3		1	0	3	9
8:00	1		0	0	0	0	1	0		1	0	0	3
8:15	0		0	0	0	0	0	0		0	0	0	0
10.00	_	1							1				
16:30	0		0	0	0	0	0	0		2	0	5	7
16:45	0		1	0	0	0	1	3		0	0	1	6
17:00	0		0	0	0	0	2	2		0	0	5	9
17:15	0		0	0	0	0	0	6		0	0	0	6
17:30	0		3	0	0	0	0	1		1	0	1	6
17:45	1		1	0	0	0	2	0		0	0	0	4
18:00	1		0	0	0	0	0	2		0	0	4	7
18:15	0		0	0	0	0	4	2		0	0	4	10
Da ele Herre	0011	007	000	WDI	MOT	WDD	NBII	NDI	NDT	ED!	FDT	EDD	
Peak Hour	SBU	SBT	SBR	WBL	WBT	WBR	NBU 3	NBL	NBT	EBL 0	EBT	EBR 9	Volume
6:30 - 7:30 6:45 - 7:45	0	0	1	0	0	0	1	0 1	0	0	0	7	13 10
6:45 - 7:45 7:00 - 8:00	0	0	2	0	0	0	2	4	0	1	0	7	16
7:15 - 8:15	1	0		0	0	0	2	4	0	2	0		15
7:30 - 8:30	1	0	1	0	0	0	2	4	0	2	0	3	13
7.30 - 6.30	<u> </u>	U	<u> </u>	U	U	U		4	U		U	3	13
Peak Hour	SBU	SBT	SBR	WBL	WBT	WBR	NBU	NBL	NBT	EBL	EBT	EBR	Volume
16:30 - 17:30	0	0	1	0	0	0	3	11	0	2	0	11	28
16:45 - 17:45		0	4	0	0	0	3	12	0	1	0	7	27
17:00 - 18:00		0	4	0	0	0	4	9	0	1	0	6	25
17:15 - 18:15		0	4	0	0	0	2	9	0	1	0	5	23
17:30 - 18:30		0	4	0	0	0	6	5	0	1	0	9	27
		- V	· ·	- V	Ŭ	Ŭ	Ŭ				Ŭ		
					Peak-l	lour Tra	ffic Volu	ımes					
Peak Hour	NBU	NBL	NBT	SBU	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Volume
7:00 - 8:00		4	0	000	02.	2	1	0	7	0	0	0	16

				Peak-Hour Traffic Volumes												
Peak Hour	NBU	NBL	NBT	SBU	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Volume			
7:00 - 8:00	2	4	0	0	0	2	1	0	7	0	0	0	16			
16:30 - 17:30	3	11	0	0	0	1	2	0	11	0	0	0	28			

				F	eak-Ho	ur Facto	r by Mo	vement					
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	PHF
7:00 - 8:00	0.333	-	#REF!	#REF!	-	0.500	0.250	-	0.583	-	-	-	0.444
16:30 - 17:30	0.458	-	#REF!	#REF!	-	0.250	0.250	-	0.550	-	-	-	0.778

		Peak-Hour Facto	or by Approach		
Peak Hour	NB	SB	EB	WB	PHF
7:00 - 8:00	0.38	0.50	0.50	=	0.44
16:30 - 17:30	0.58	0.25	0.46	-	0.78

				Heav	y Vehicl	e Percei	ntage by	Movem	ent				
Peak Hour NBL NBT NBR SBL SBT SBR EBL EBT EBR WBL WBT WBR %HV													
7:00 - 8:00	0%	0%	#REF!	#REF!	0%	0%	0%	0%	0%	0%	0%	0%	0%
16:30 - 17:30	0 - 17:30 0% 0% #REF! #REF! 0% 0% 0% 0% 0% 0% 0% 0% 0%												

		Heavy Vehicle Perce	ntage by Approach										
Peak Hour	NB SB EB WB %HV												
7:00 - 8:00	#REF!	#REF!	0%	0%	0%								
16:30 - 17:30	#RFF!	#REF! #REF! 0% 0% 0%											

/Wallridge Drive and Harris Road AM and PM Peak Hour Traffic Count Count Performed: Tuesday, October 30, 2018

	Wal	Ilridge D	rive	Ha	arris Ro	ad		0		H	arris Ro	ad	Intersection
	Sc	outhbou	nd	V	/estboui	nd	N	orthbou	nd	Ш	astbour	nd	Volume
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	volume
6:30	1	0	5	0	35	3	0	0	0	4	37	0	85
6:45	7	0	8	0	39	6	0	0	0	13	99	0	172
7:00	22	0	21	0	37	9	0	0	0	13	105	0	207
7:15	2	0	10	0	46	6	0	0	0	15	32	0	111
7:30	3	0	19	0	30	14	0	0	0	9	30	0	105
7:45	6	0	16	0	42	7	0	0	0	12	31	0	114
8:00	2	0	13	0	48	10	0	0	0	20	27	0	120
8:15	2	0	16	0	46	9	0	0	0	27	42	0	142
								•					
16:30	18	0	36	0	24	9	0	0	0	33	42	0	162
16:45	10	0	37	0	35	16	0	0	0	37	52	0	187
17:00	15	0	27	0	48	15	0	0	0	26	50	0	181
17:15	14	0	33	0	46	9	0	0	0	39	48	0	189
17:30	18	0	23	0	40	16	0	0	0	32	34	0	163
17:45	15	0	33	0	47	22	0	0	0	49	57	0	223
18:00	16	0	35	0	22	12	0	0	0	26	52	0	163
18:15	26	0	32	0	35	11	0	0	0	35	33	0	172
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
6:30 - 7:30	32	0	44	0	157	24	0	0	0	45	273	0	575
6:45 - 7:45	34	0	58	0	152	35	0	0	0	50	266	0	<u>595</u>
7:00 - 8:00	33	0	66	0	155	36	0	0	0	49	198	0	537
7:15 - 8:15	13	0	58	0	166	37	0	0	0	56	120	0	450
7:30 - 8:30	13	0	64	0	166	40	0	0	0	68	130	0	481
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
16:30 - 17:30	56L	0	133	0	153	49	0	0	0	135	192	0	719
16:45 - 17:45	57	0	120	0	169	56	0	0	0	134	184	0	719
17:00 - 18:00	62	0	116	0	181	62	0	0	0	146	189	0	756
17:15 - 18:15	63	0	124	0	155	59	0	0	0	146	191	0	738
17:30 - 18:30	75	0	123	0	144	61	0	0	0	140	176	0	730
17.50 - 10.50	13		120	U	177	1 01	U		U	174	170	U	121
					Peak-l	lour Tra	ffic Volu	ımes					
Peak Hour	NRI	NRT	NBR	SBI	SBT		FBI	FRT	FBR	WBI	WRT	WBR	Volume

					Peak-l	lour Tra	ffic Volu	ımes					
Peak Hour	Peak Hour NBL NBT NBR SBL SBT SBR EBL EBT EBR WBL WBT WBR Volume												
6:45 - 7:45	0	0	0	34	0	58	50	266	0	0	152	35	595
17:00 - 18:00	0	0	0	62	0	116	146	189	0	0	181	62	756

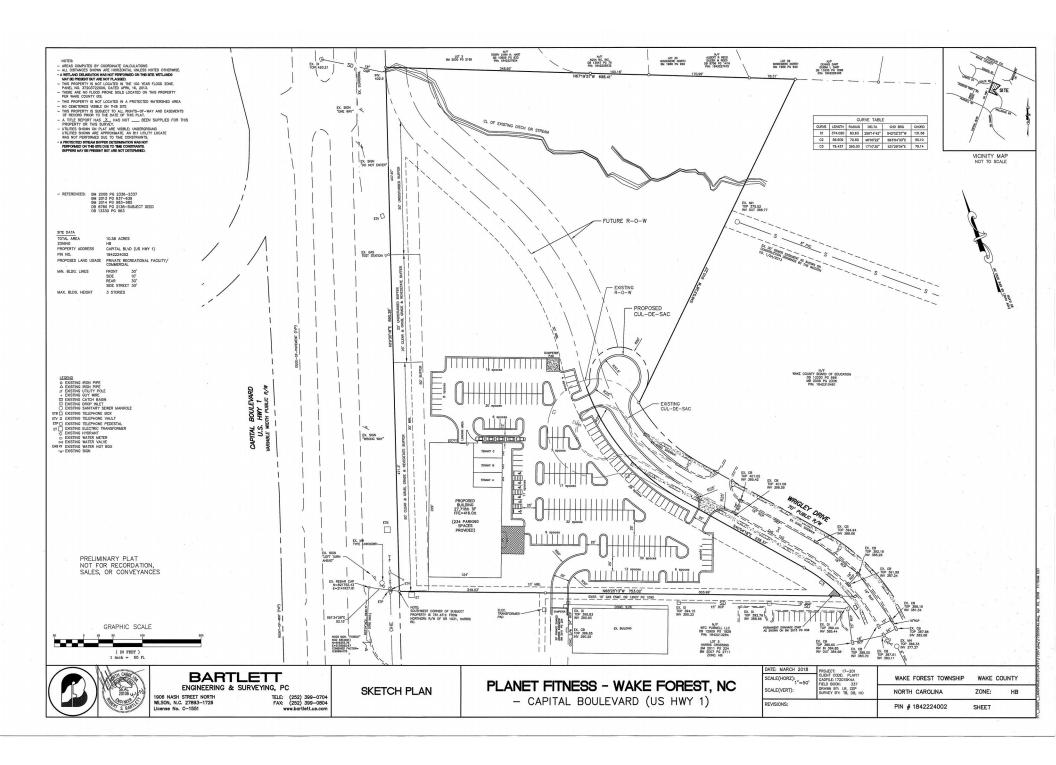
				F	eak-Ho	ur Facto	r by Mo	vement					
Peak Hour	Peak Hour NBL NBT NBR SBL SBT SBR EBL EBT EBR WBL WBT WBR PHF												
6:45 - 7:45	45 - 7:45 0.386 - 0.690 0.833 0.633 0.826 0.625 0.719												
17:00 - 18:00	7:00 - 18:00 - - - 0.861 - 0.879 0.582 0.829 - - 0.943 0.705 0.848												

		Peak-Hour Facto	or by Approach		
Peak Hour	NB	SB	EB	WB	PHF
6:45 - 7:45	-	0.54	0.67	0.90	0.72
17:00 - 18:00	=	0.93	0.79	0.88	0.85

				Heav	y Vehicl	e Percei	ntage by	Movem	ent				
Peak Hour NBL NBT NBR SBL SBT SBR EBL EBT EBR WBL WBT WBR %HV													
6:45 - 7:45	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
17:00 - 18:00	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

		Heavy Vehicle Perce	ntage by Approach														
Peak Hour	NB	SB	EB	WB	%HV												
6:45 - 7:45	0%	0% 0% 0% 0%															
17:00 - 18:00	0%	0%	0%	0%	00 - 18:00												

Appendix D: Approved Development Data



		APP DE	VELOPME	NT - PLA	NET FITN	IESS						
			Table 1 - T	rip Gene	ration							
Land Use	Land Use	Inten	oitu		Daily		Al	/I Peak Ho	ur	PI	/I Peak Ho	ur
Code	Land OSe	inten	isity	Total	ln	Out	Total	ln	Out	Total	In	Out
492	Health/Fitness Club ²	27,718	s.f.	710	355	355	36	18	18	106	60	46

K:\RAL_TPTO_Traffic\018772001 Devon Square\T4 - Analysis\[AppDev-PlanetFitness.xls]Trip Gen

N/S Street: US 1 (Capital Boulevard)

E/W Street: Harris Road/Purnell Road

Annual Growth Rate: 3.0%

Growth Factor: 0.125509

Buildout Year: 2018

AMPEAN HOUR

AM PEAK HOUR AM PHF = 0.86

				A.	MI I III - 0	.00							
		Purnell Road			Harris Road			US 1 (Capi	tal Boulevard)		US 1	(Capital Boul	evard)
		Eastbound			Westbound			Nort	hbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	U-Turn	Left	Through	Right	Left	Through	Right
Project Traffic													
Percent Assignment Inbound	0%	5%	0%	0%	0%	0%	0%	0%	15%	5%	20%	0%	0%
Inbound Project Traffic	0	1	0	0	0	0	0	0	3	1	4	0	0
Percent Assignment Outbound	0%	0%	0%	20%	5%	0%	0%	0%	0%	0%	0%	0%	0%
Outbound Project Traffic	0	0	0	4	1	0	0	0	0	0	0	0	0
Total Project Traffic	0	1	0	4	1	0	0	0	3	1	4	0	0

PM PEAK HOUR PM PHF = .99

		Purnell Road			Harris Road			US 1 (Capita	l Boulevard)		US 1	(Capital Boul	evard)
		Eastbound			Westbound			North	bound			Southbound	
Description	Left	Through	Right	Left	Through	Right	U-Turn	Through	Through	Right	Left	Through	Right
Project Traffic													
Percent Assignment Inbound	0%	5%	0%	0%	0%	0%	0%	0%	15%	5%	20%	0%	0%
Inbound Project Traffic	0	3	0	0	0	0	0	0	9	3	12	0	0
Percent Assignment Outbound	0%	0%	0%	20%	5%	0%	0%	0%	0%	0%	0%	0%	0%
Outbound Project Traffic	0	0	0	9	2	0	0	0	0	0	0	0	0
Total Project Traffic	0	3	0	9	2	0	0	0	9	3	12	0	0

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 18
 18
 60
 46
 APP DEVELOPMENT - PLANET FITNESS Project: Location: Wake Forest, NC
Ct. Date 10/30/2018
N/S Street: US 1 (Capital Bouelvard)

Annual Growth Rate: 3.0% Growth Factor: 0.125509 Existing Year: 2018
Buildout Year: 2022 E/W Street: Club Villas Drive AM PEAK HOUR AM PHF =

					ANI I III -								
		Club Villas Drive			Club Villas Dri	ve		US 1 (Capit	tal Bouelvard)		US 1	(Capital Boue	lvard)
		Eastbound			Westbound			Nort	hbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Through	Right
Project Traffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	20%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	0	4	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	20%	0%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	0	4	0
Total Project Traffic	0	0	0	0	0	0	0	0	4	0	0	4	0

PM PEAK HOUR PM PHF =

	(Club Villas Drive			lub Villas Dri	ve		US 1 (Capit	tal Bouelvard)		US 1	(Capital Boue	lvard)
		Eastbound			Westbound			Nort	hbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left		Through	Right	Left	Through	Right
Project Traffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	20%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	0	12	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	20%	0%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	0	9	0
Total Project Traffic	0	0	0	0	0	0	0	0	12	0	0	9	0

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 18
 18
 60
 46
 Project: APP DEVELOPMENT - PLANET FITNESS
Location: Wake Forest, NC
10/30/2018
N/S Street: E/W Street: Wallridge Drive
Harris Road

Annual Growth Rate:	3.0%	Existing Year:	2018
Growth Factor:	0.125509	Buildout Year:	2022

AM PEAK HOUR AM PHF = 0.72

		Harris Road		Harris Road			Wallridge Drive			Wallridge Drive		
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Project Traffic												
Percent Assignment Inbound	30%	0%	0%	0%	0%	30%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	5	0	0	0	0	5	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	30%	0%	25%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	5	0	5
Total Project Traffic	5	0	0	0	0	5	0	0	0	5	0	5

PM PEAK HOUR PM PHF = 0.85

		Harris Road		Harris Road			Wallridge Drive				Wallridge Driv	e
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Project Traffic												
Percent Assignment Inbound	30%	0%	0%	0%	0%	30%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	18	0	0	0	0	18	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	30%	0%	25%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	14	0	12
Total Project Traffic	18	0	0	0	0	18	0	0	0	14	0	12

			AM In	AM Out	PM In	PM Out
Project:	APP DEVELOPMENT - PLANET FITNESS	Net New Trips:	18	18	60	46
Location:	Wake Forest, NC	_				
Ct. Date	Balanced					
N/S Street:	Site Driveway	Annual Growth Rate:	3.0%	Exist	ting Year:	2018
E/W Street:	Harris Road	Growth Factor:	0.125509	Build	lout Year:	2022

AM PEAK HOUR AM PHF = 0.90

		Harris Road			Harris Road		Site Driveway			Site Driveway		
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	30%	0%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	5	0	0	0	0	0	0	0
Percent Assignment Outbound	0%	30%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Outbound Project Traffic	0	5	0	0	0	0	0	0	0	0	0	0
Total Project Traffic	0	5	0	0	5	0	0	0	0	0	0	0

PM PEAK HOUR PM PHF = 0.90

	Harris Road			Harris Road			Site Driveway	,	Site Driveway			
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	30%	0%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	18	0	0	0	0	0	0	0
Percent Assignment Outbound	0%	30%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Outbound Project Traffic	0	14	0	0	0	0	0	0	0	0	0	0
Total Project Traffic	0	14	0	0	18	0	0	0	0	0	0	0

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 18
 18
 60
 46
 Project: APP DEVELOPMENT - PLANET FITNESS
Location: Wake Forest, NC
Ct. Date
N/S Street: US 1 (Capital Bouelvard) Annual Growth Rate: 3.0%

Existing Year: 2018 Growth Factor: 0.125509 **Buildout Year:** 2022

AM PEAK HOUR $\mathbf{AM}\ \mathbf{PHF} = \mathbf{0.90}$

		Site Driveway				US 1	(Capital Boue	lvard)	US 1 (Capital Bouelvard)			
		Eastbound		Westbound				Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Project Traffic Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	20%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	4	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	20%	0%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	4	0
Total Project Traffic	0	0	0	0	0	0	0	4	0	0	4	0

PM PEAK HOUR PM PHF = 0.90

		Site Driveway	,	Site Driveway			US 1 (Capital Bouelvard)			US 1	(Capital Bouel	lvard)	
		Eastbound		Westbound				Northbound			Southbound		
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
Project Traffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	20%	0%	0%	0%	0%	
Inbound Project Traffic	0	0	0	0	0	0	0	12	0	0	0	0	
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	20%	0%	
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	9	0	
Total Project Traffic	0	0	0	0	0	0	0	12	0	0	9	0	

E/W Street: Site Driveway

TRAFFIC IMPACT ANALYSIS

FOR

GLEN OAKS

LOCATED

IN

WAKE FOREST, NORTH CAROLINA

Prepared For: GREENPOINTE, LLC 7201 Creedmoor Road, Suite 140 Raleigh, NC 27613

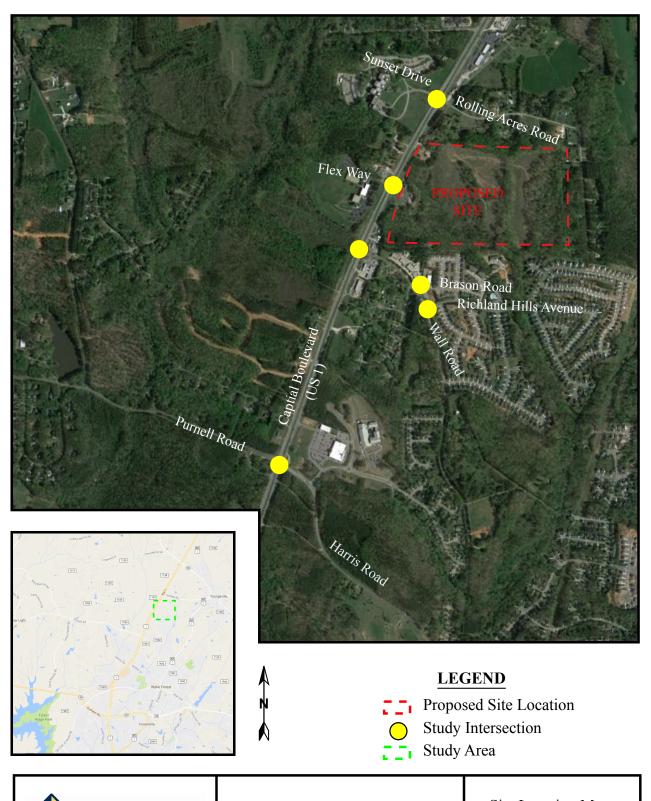
Prepared By:
Ramey Kemp & Associates, Inc.
5808 Faringdon Place, Suite 100
Raleigh, NC 27609
License #C-0910

December 2017

SEAL 0392512

Prepared By: <u>DL</u>

Reviewed By: <u>JR</u>





Glen Oaks Wake Forest, NC Site Location Map

Scale: Not to Scale

Figure 1

4. SITE TRIP GENERATION AND DISTRIBUTION

4.1. Trip Generation

The proposed development was studied in two phases; Phase 1 is expected to be completed in 2019 with 108 single-family homes and 33 townhomes, while Phase 2 (full build-out) is expected to be completed in 2022 with a total of 225 single-family homes and 73 townhomes. Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 9th Edition. Tables 1 and 2 provides a summary of the trip generation potential for the site under Phase 1 and Phase 2, respectively.

PM Peak Hour **AM Peak Hour Daily Land Use** Trips (vph) Trips (vph) Intensity Traffic (ITE Code) (vpd) Enter Exit Enter Exit Single Family Detached Housing 108 1,030 20 61 68 40 dwellings (210)Residential Condo / Townhouse 33 190 3 12 12 6 dwellings (230)23 73 80 **Total Trips** 1,220 46

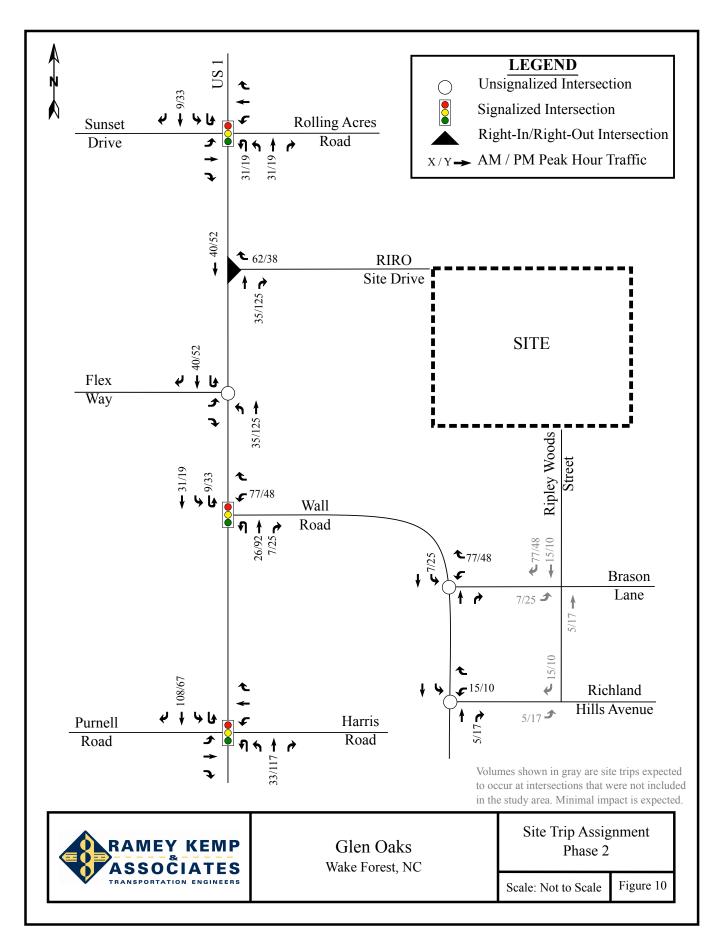
Table 1: Trip Generation Summary -Phase 1

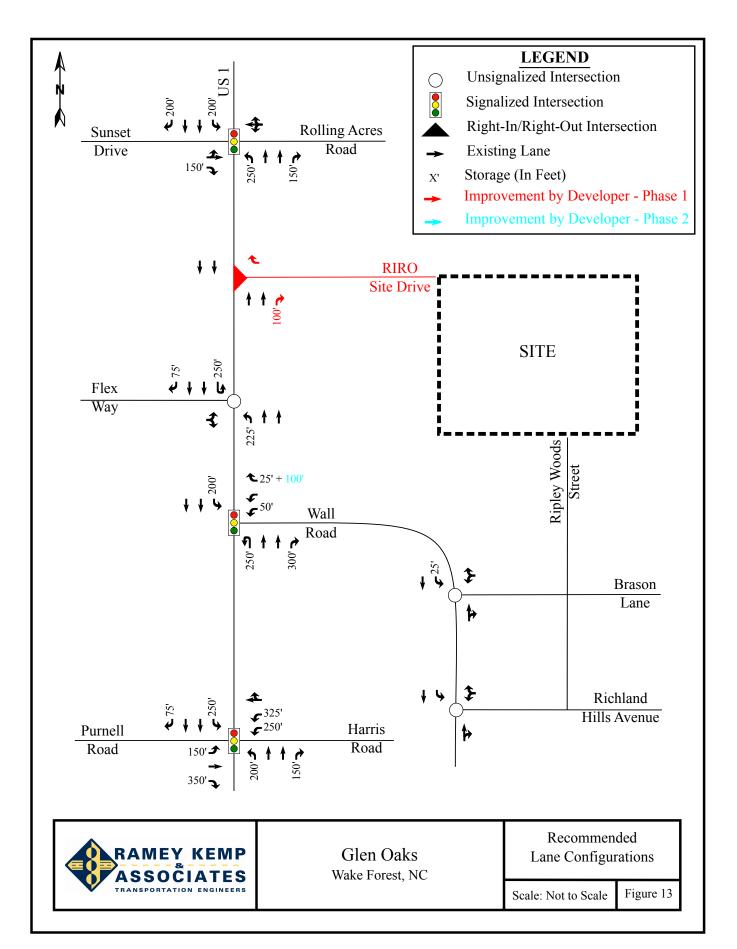
It is estimated that once Phase 1 is complete, the proposed development will generate approximately 1,220 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 96 trips (23 entering and 73 exiting) will occur during the AM peak hour and 126 (80 entering and 46 exiting) will occur during the PM peak hour.

Table 2: Trip Generation Summary - Phase 2

Land Use (ITE Code)	Intensity	Daily Traffic	AM Pea Trips		PM Pea Trips	
(TTE Code)		(vpd)	Enter	Exit	Enter	Exit
Single Family Detached Housing (210)	225 dwellings	2,140	42	127	142	83
Residential Condo / Townhouse (230)	73 dwellings	430	5	27	25	13
Total Trips	2,570	47	154	167	96	







Appendix E: Intersection Spreadsheets

Project: Devon Square

Location: Wake Forest, NC

Ct. Date 10/30/2018

N/S Street: US 1 (Capital Boulevard)

E/W Street: Harris Road/Purnell Road

	AM In	AM Out	PM In	PM Out	
Net New Trips:	38	114	126	75	

Annual Growth Rate: 3.0% Existing Year: 2018 Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.86

					A.	WPHF = U	.00							
		Purnell Road Harris Road US 1 (Capital Boulevard) Eastbound Westbound Northbound									US 1	(Capital Bould Southbound	evard)	
Descrip	tion	Left	Through	Right	Left	Through	Right	U-Turn	Left	Through	Right	Left	Through	Right
2018	Traffic Count	64	159	88	129	67	14	0	53	925	126	31	1502	56
	alancing	0	0	0	0	0	0	0	0	0	0	0	0	0
2018	Existing Traffic	64	159	88	129	67	14	0	53	925	126	31	1502	56
Growth 1	Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022	Background Growth	8	20	11	16	8	2	0	7	116	16	4	189	7
Commit	tted Projects													
Planet F	itness	0	1	0	4	1	0	0	0	3	1	4	0	0
Glen Oa	ks Residential	0	0	0	0	0	0	0	33	33	0	0	108	0
Total C	ommitted Traffic	0	1	0	4	1	0	0	33	36	1	4	108	0
2022	Background Traffic	72	180	99	149	76	16	0	93	1077	143	39	1799	63
Project														
	Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	5%	10%	0%
Inbound	Project Traffic	0	0	0	0	0	0	0	0	0	0	2	4	0
Percent A	Assignment Outbound	0%	0%	0%	35%	0%	0%	30%	0%	15%	0%	0%	0%	0%
Outboun	d Project Traffic	0	0	0	40	0	0	34	0	17	0	0	0	0
Total Pi	roject Traffic	0	0	0	40	0	0	34	0	17	0	2	4	0
2022	Buildout Total	72	180	99	189	76	16	34	93	1094	143	41	1803	63
Percent 1	Impact (Approach)		0.0%			14.2%			3.	7%		1	0.3%	

Overall Percent Impact 2.5%

PM PEAK HOUR PM PHF = .99

				1	MPHF = .	"							
		Purnell Road Eastbound			Harris Road Westbound				il Boulevard) bound		US 1	(Capital Boule Southbound	vard)
Description	Left	Through	Right	Left	Through	Right	U-Turn	Through	Through	Right	Left	Through	Right
2010 T 87 C /	100	104	107	140	110	20		00	1500	145		1457	44
2018 Traffic Count Count Balancing	100	104	107	148 0	113 0	28	0	98 0	1566	145 0	69	1457 0	44 0
2018 Existing Traffic	100	104	107	148	113	28	0	98	1566	145	69	1457	44
2010 Embung Trume	100	101	107	110		20		,,,	1500	110	0,	1137	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	13	13	13	19	14	4	0	12	197	18	9	183	6
Committed Projects													
Planet Fitness	0	3	0	9	2	0	0	0	9	3	12	0	0
Glen Oaks Residential	0	0	0	0	0	0	0	117	117	0	0	67	0
Total Committed Traffic	0	3	0	9	2	0	0	117	126	3	12	67	0
2022 Background Traffic	113	120	120	176	129	32	0	227	1889	166	90	1707	50
Project Traffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	5%	10%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	0	0	0	6	13	0
Percent Assignment Outbound	0%	0%	0%	35%	0%	0%	30%	0%	15%	0%	0%	0%	0%
Outbound Project Traffic	0	0	0	26	0	0	23	0	11	0	0	0	0
Total Project Traffic	0	0	0	26	0	0	23	0	11	0	6	13	0
2022 Buildout Total	113	120	120	202	129	32	23	227	1900	166	96	1720	50
Percent Impact (Approach)		0.0%			7.2%			1.5	5%			1.0%	
Overall Percent Impact	1.6%												

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 38
 114
 126
 75
 Project: Devon Square Location: Ct. Date N/S Street: Wake Forest, NC 10/30/2018 US 1 (Capital Bouelvard) Annual Growth Rate: 3.0% Growth Factor: 0.125509 Existing Year: 2018
Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.90

Club Villas Drive US 1 (Capital Bouelvard) US 1 (Capital Bouelvard)														
		(Club Villas Dri Eastbound	ve			al Bouelvard) abound		US 1	(Capital Bouel Southbound	lvard)			
Descrip	otion	Left	Through	Right	Left	Westbound Through	Right	U-Turn	Left	Through	Right	U-Turn	Through	Right
2018	Traffic Count	1	0	7	0	0	0	2	4	0	0	0	0	2
	Balancing	0	0	0	0	0	0	0	0	1103	0	0	1717	0
2018	Existing Traffic	1	0	7	0	0	0	2	4	1103	0	0	1717	2
Growth	Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022	Background Growth	0	0	1	0	0	0	0	1	138	0	0	215	0
Commi	itted Projects													
Planet F		0	0	0	0	0	0	0	0	4	0	0	4	0
Glen Oa	Glen Oaks Residential		0	0	0	0	0	0	0	66	0	0	108	0
Total C	Committed Traffic	0	0	0	0	0	0	0	0	70	0	0	112	0
2022	Background Traffic	1	0	8	0	0	0	2	5	1311	0	0	2044	2
	Traffic													
	Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	65%	0%	10%	0%	0%
Inbound	l Project Traffic	0	0	0	0	0	0	0	0	25	0	4	0	0
Percent	Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	65%	0%
Outbour	nd Project Traffic	0	0	0	0	0	0	0	0	0	0	0	74	0
Total P	roject Traffic	0	0	0	0	0	0	0	0	25	0	4	74	0
2022	Buildout Total	1	0	8	0	0	0	2	5	1336	0	4	2118	2
Percent	Impact (Approach)		0.0%			-			1.	9%			3.7%	

Overall Percent Impact 3.0%

E/W Street: Club Villas Drive

PM PEAK HOUR PM PHF = 0.90

2022 Background Growth														
Description Left Through Right Righ		(Club Villas Driv	ve					US 1 (Capit	al Bouelvard)		US 1	(Capital Boue	lvard)
2018 Traffic Count 2 0 11 0 0 0 0 3 11 0 0 0 0 0 1711			Eastbound			Westbound			North	bound		ĺ	Southbound	
Count Balancing 0 0 0 0 0 0 0 0 0 1807 0 0 1711 2018 Existing Traffic 2 0 11 0 0 0 0 3 11 1807 0 0 1711 Growth Factor (0.03 per year) 0.126 0.12	Description	Left	Through	Right	Left	Through	Right	Left		Through	Right	Left	Through	Right
Count Balancing 0 0 0 0 0 0 0 0 0 1807 0 0 1711 2018 Existing Traffic 2 0 11 0 0 0 0 3 11 1807 0 0 1711 Growth Factor (0.03 per year) 0.126 0.12	· ·													
2018 Existing Traffic 2 0 11 0 0 0 3 11 1807 0 0 1711 Growth Factor (0.03 per year) 0.126 <td></td> <td>1</td>														1
Growth Factor (0.03 per year) 0.126	Count Balancing	0	0	0	0	0	0	0	0	1807	0	0	1711	0
2022 Background Growth 0 0 1 0 0 0 1 227 0 0 215 Committed Projects Planet Fitness 0 0 0 0 0 0 0 12 0 0 9 Glen Oaks Residential 0 0 0 0 0 0 0 234 0 0 67 Total Committed Traffic 0 0 0 0 0 0 246 0 0 76 2022 Background Traffic 2 0 12 0 0 3 12 2280 0 0 2002 Project Traffic Percent Assignment Inbound 0%	2018 Existing Traffic	2	0	11	0	0	0	3	11	1807	0	0	1711	1
2022 Background Growth 0 0 1 0 0 0 1 227 0 0 215 Committed Projects Planet Fitness 0 0 0 0 0 0 0 12 0 0 9 Glen Oaks Residential 0 0 0 0 0 0 0 234 0 0 67 Total Committed Traffic 0 0 0 0 0 0 246 0 0 76 2022 Background Traffic 2 0 12 0 0 3 12 2280 0 0 2002 Project Traffic Percent Assignment Inbound 0%	Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
Planet Fitness 0 0 0 0 0 0 0 0 0		0	0	1	0	0	0	0	1	227	0	0	215	0
Planet Fitness	Committed Projects													
Glen Oaks Residential 0 0 0 0 0 0 0 0 0 234 0 0 67 Total Committed Traffic 0 0 0 0 0 0 0 0 0 246 0 0 0 76 2022 Background Traffic 2 0 12 0 0 0 3 12 280 0 0 2002 Project Traffic Percent Assignment Inbound Inbound Project Traffic 0 0 0 0 0 0 0 0 0 0 0 82 0 13 0 Percent Assignment Outbound 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%	Planet Fitness	0	0	0	0	0	0	0	0	12	0	0	9	0
2022 Background Traffic 2 0 12 0 0 3 12 2280 0 0 2002 Project Traffic Percent Assignment Inbound 0% 0	Glen Oaks Residential	0		0	0	0	0	0	0		0	0	67	0
Project Traffic Project Traffic Project Traffic O% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%	Total Committed Traffic	0	0	0	0	0	0	0	0	246	0	0	76	0
Percent Assignment Inbound 0% <th< td=""><td>2022 Background Traffic</td><td>2</td><td>0</td><td>12</td><td>0</td><td>0</td><td>0</td><td>3</td><td>12</td><td>2280</td><td>0</td><td>0</td><td>2002</td><td>1</td></th<>	2022 Background Traffic	2	0	12	0	0	0	3	12	2280	0	0	2002	1
Inbound Project Traffic 0 0 0 0 0 0 0 82 0 13 0 Percent Assignment Outbound 0%	Project Traffic													
Percent Assignment Outbound 0% <t< td=""><td>Percent Assignment Inbound</td><td>0%</td><td>0%</td><td>0%</td><td>0%</td><td>0%</td><td>0%</td><td>0%</td><td>0%</td><td>65%</td><td>0%</td><td>10%</td><td>0%</td><td>0%</td></t<>	Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	65%	0%	10%	0%	0%
Outbound Project Traffic 0 0 0 0 0 0 0 0 0 0 0 49	nbound Project Traffic	0	0	0	0	0	0	0	0	82	0	13	0	0
Outbound Project Traffic 0 0 0 0 0 0 0 0 0 0 0 49	Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	65%	0%
		0	0	0	0	0	0	0	0	0	0	0	49	0
Total Project Traffic 0 0 0 0 0 0 0 0 82 0 13 49	Total Project Traffic	0	0	0	0	0	0	0	0	82	0	13	49	0
2022 Buildout Total 2 0 12 0 0 0 3 12 2362 0 13 2051	2022 Buildout Total	2		12	0	0	0	3			0	13		1
Percent Impact (Approach) 0.0% - 3.4% 3.0%			0.0%			-			3.	4%			3.0%	
Overall Percent Impact 3.2%	Overall Percent Impact	3.2%		•										

Project: Devon Square
Location: Wake Forest, NC
Ct. Date 10/30/2018
N/S Street: Wallridge Drive
E/W Street: Harris Road

	AM In	AM Out	PM In	PM Out
Net New Trips:	38	114	126	75

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.72

_					73.	$\mathbf{M} \mathbf{P} \mathbf{\Pi} \mathbf{r} = 0.$	14						
			Harris Road Eastbound			Harris Road Westbound		,	Wallridge Drive Northbound	e	,	Wallridge Drive Southbound	e
Description		Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
				_	_				_				
2018 Traffic C	ount	50	266	0	0	152	35	0	0	0	34	0	58
Count Balancing		0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing	Fraffic	50	266	0	0	152	35	0	0	0	34	0	58
Growth Factor (0.03	per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Backgrou	ind Growth	6	33	0	0	19	4	0	0	0	4	0	7
Committed Projects	s												
Planet Fitness		5	0	0	0	0	5	0	0	0	5	0	5
Glen Oaks Residentia	al	0	0	0	0	0	0	0	0	0	0	0	0
Total Committed T	raffic	5	0	0	0	0	5	0	0	0	5	0	5
2022 Backgrou	ınd Traffic	61	299	0	0	171	44	0	0	0	43	0	70
Project Traffic													
Percent Assignment	Inbound	0%	5%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Inbound Project Traf	ffic	0	2	0	0	0	0	0	0	0	0	0	0
Percent Assignment	Outbound	0%	0%	0%	0%	35%	0%	0%	0%	0%	0%	0%	0%
Outbound Project Tr		0	0	0	0	40	0	0	0	0	0	0	0
Total Project Traff	ic	0	2	0	0	40	0	0	0	0	0	0	0
2022 Buildout	Total	61	301	0	0	211	44	0	0	0	43	0	70
Percent Impact (App	roach)		0.6%	•		15.7%	•		-			0.0%	•

Overall Percent Impact 5.8

PM PEAK HOUR PM PHF = 0.85

Third wood													
		Harris Road			Harris Road			Wallridge Driv	e	,	Wallridge Driv	e	
		Eastbound			Westbound			Northbound			Southbound		
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
2018 Traffic Count	146	189	0	0	181	62	0	0	0	62	0	116	
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0	
2018 Existing Traffic	146	189	0	0	181	62	0	0	0	62	0	116	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022 Background Growth	18	24	0	0	23	8	0	0	0	8	0	15	
Committed Projects													
Planet Fitness	18	0	0	0	0	18	0	0	0	14	0	12	
Glen Oaks Residential	0	0	0	0	0	0	0	0	0	0	0	0	
Total Committed Traffic	18	0	0	0	0	18	0	0	0	14	0	12	
2022 Background Traffic	182	213	0	0	204	88	0	0	0	84	0	143	
Project Traffic													
Percent Assignment Inbound	0%	5%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
Inbound Project Traffic	0	6	0	0	0	0	0	0	0	0	0	0	
Percent Assignment Outbound	0%	0%	0%	0%	35%	0%	0%	0%	0%	0%	0%	0%	
Outbound Project Traffic	0	0	0	0	26	0	0	0	0	0	0	0	
Total Project Traffic	0	6	0	0	26	0	0	0	0	0	0	0	
2022 Buildout Total	182	219	0	0	230	88	0	0	0	84	0	143	
Percent Impact (Approach)		1.5%			8.2%			-			0.0%		

Overall Percent Impact 3.4%

Project: Devon Square
Location: Wake Forest, NC
Ct. Date
N/S Street: Balanced
Site Driveway
Harris Road

	AM In	AM Out	PM In	PM Out
et New Trips:	38	114	126	75

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.90

				А	1VI I III — U	.70						
		Harris Road			Site Driveway							
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing	0	300	0	0	187	0	0	0	0	0	0	0
2018 Existing Traffic	0	300	0	0	187	0	0	0	0	0	0	0
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	38	0	0	23	0	0	0	0	0	0	0
Committed Projects												
Planet Fitness	0	5	0	0	5	0	0	0	0	0	0	0
Glen Oaks Residential	0	0	0	0	0	0	0	0	0	0	0	0
Total Committed Traffic	0	5	0	0	5	0	0	0	0	0	0	0
2022 Background Traffic	0	343	0	0	215	0	0	0	0	0	0	0
Project Traffic												
Percent Assignment Inbound	0%	0%	5%	20%	0%	0%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	2	7	0	0	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	35%	0%	20%	0%	0%	0%
Outbound Project Traffic	0	0	0	0	0	0	40	0	23	0	0	0
Total Project Traffic	0	0	2	7	0	0	40	0	23	0	0	0
2022 Buildout Total	0	343	2	7	215	0	40	0	23	0	0	0
Percent Impact (Approach)		0.6%			3.2%			100.0%			-	

Overall Percent Impact 11.4%

PM PEAK HOUR PM PHF = 0.90

H. D. L. C. D.													
		Harris Road			Harris Road			Site Driveway	/		Site Driveway	1	
		Eastbound			Westbound			Northbound			Southbound		
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
	_			_	_		_	_	_	_		_	
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
Count Balancing	0	251	0	0	243	0	0	0	0	0	0	0	
2018 Existing Traffic	0	251	0	0	243	0	0	0	0	0	0	0	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022 Background Growth	0	32	0	0	30	0	0	0	0	0	0	0	
Committed Projects													
Planet Fitness	0	14	0	0	18	0	0	0	0	0	0	0	
Glen Oaks Residential	0	0	0	0	0	0	0	0	0	0	0	0	
Total Committed Traffic	0	14	0	0	18	0	0	0	0	0	0	0	
2022 Background Traffic	0	297	0	0	291	0	0	0	0	0	0	0	
Project Traffic													
Percent Assignment Inbound	0%	0%	5%	20%	0%	0%	0%	0%	0%	0%	0%	0%	
Inbound Project Traffic	0	0	6	25	0	0	0	0	0	0	0	0	
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	35%	0%	20%	0%	0%	0%	
Outbound Project Traffic	0	0	0	0	0	0	26	0	15	0	0	0	
Total Project Traffic	0	0	6	25	0	0	26	0	15	0	0	0	
2022 Buildout Total	0	297	6	25	291	0	26	0	15	0	0	0	
Percent Impact (Approach)		2.0%			7.9%			100.0%			-		

Overall Percent Impact 10.9%

Project: Devon Square Net New Trips:

Location: Wake Forest, NC

Ct. Date
N/S Street: Balanced
US 1 (Capital Bouelvard)

E/W Street: Site Driveway

Annual Growth Rate:

Growth Factor:

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

 AM In
 AM Out
 PM In
 PM Out

 38
 114
 126
 75

AM PEAK HOUR AM PHF = 0.90

Site Driveway Site Driveway US 1 (Capital Bouelvard) US 1 (Capital Bouelvard)													
			Site Driveway	•		Site Driveway		US 1	(Capital Boue	lvard)	US 1	(Capital Bouel	vard)
			Eastbound			Westbound			Northbound			Southbound	
Description		Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count		0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing		0	0	0	0	0	0	0	1104	0	0	1719	0
2018 Existing Traffic		0	0	0	0	0	0	0	1104	0	0	1719	0
Growth Factor (0.03 per year)		0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Grov	vth	0	0	0	0	0	0	0	139	0	0	216	0
Committed Projects													
Planet Fitness		0	0	0	0	0	0	0	4	0	0	4	0
Glen Oaks Residential		0	0	0	0	0	0	0	66	0	0	108	0
Total Committed Traffic		0	0	0	0	0	0	0	70	0	0	112	0
2022 Background Traff	ïc	0	0	0	0	0	0	0	1313	0	0	2047	0
Project Traffic													
Percent Assignment Inbound		0%	0%	0%	0%	0%	0%	0%	0%	75%	0%	10%	0%
Inbound Project Traffic		0	0	0	0	0	0	0	0	29	0	4	0
Percent Assignment Outbound	I	0%	0%	0%	0%	0%	45%	0%	0%	0%	0%	65%	0%
Outbound Project Traffic		0	0	0	0	0	51	0	0	0	0	74	0
Total Project Traffic		0	0	0	0	0	51	0	0	29	0	78	0
2022 Buildout Total		0	0	0	0	0	51	0	1313	29	0	2125	0
Percent Impact (Approach)			-			100.0%			2.2%			3.7%	

Overall Percent Impact 4.

PM PEAK HOUR PM PHF = 0.90

	Site Driveway Site Driveway US 1 (Capital Bouelvard) US 1 (Capital Bouelvard)													
			Site Driveway	1		Site Driveway		US 1	(Capital Boue	lvard)	US 1	(Capital Boue	lvard)	
			Eastbound			Westbound			Northbound			Southbound		
Descrip	tion	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
2018	Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
	Balancing	0	0	0	0	0	0	0	1809	0	0	1712	0	
2018	Existing Traffic	0	0	0	0	0	0	0	1809	0	0	1712	0	
Growth	Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022	Background Growth	0	0	0	0	0	0	0	227	0	0	215	0	
Commi	tted Projects													
Planet F	itness	0	0	0	0	0	0	0	12	0	0	9	0	
Glen Oa	iks Residential	0	0	0	0	0	0	0	234	0	0	67	0	
Total C	committed Traffic	0	0	0	0	0	0	0	246	0	0	76	0	
2022	Background Traffic	0	0	0	0	0	0	0	2282	0	0	2003	0	
Project	Traffic													
Percent	Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	75%	0%	10%	0%	
Inbound	l Project Traffic	0	0	0	0	0	0	0	0	95	0	13	0	
Percent	Assignment Outbound	0%	0%	0%	0%	0%	45%	0%	0%	0%	0%	65%	0%	
Outbour	nd Project Traffic	0	0	0	0	0	34	0	0	0	0	49	0	
Total P	roject Traffic	0	0	0	0	0	34	0	0	95	0	62	0	
2022	Buildout Total	0	0	0	0	0	34	0	2282	95	0	2065	0	
Percent	Percent Impact (Approach) -		-		1	100.0%	·		4.0%			3.0%		

Overall Percent Impact 4.3%

Appendix F:
Synchro Output:
Existing (2018)

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
Lane Configurations
Traffic Volume (vph) 64 159 88 129 67 14 53 925 126 31 1502 56 Future Volume (vph) 64 159 88 129 67 14 53 925 126 31 1502 56 Ideal Flow (vphpl) 1900
Future Volume (vph) 64 159 88 129 67 14 53 925 126 31 1502 56 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 190
Ideal Flow (vphpl) 1900
Lane Width (ft) 12 12 12 12 12 12 12 12 12 12 12 12 12
Storage Length (ft) 150 350 235 0 300 150 150 65
Storage Length (ft) 150 350 235 0 300 150 150 65 Storage Lanes 1 1 2 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Storage Lanes 1 1 2 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Taper Length (ft) 100 200 100 100 3539 1583 1770 3539 1583 1770 3539 1583 1770 3539 1583 1770 3539 1583 1770 3539 1583 1770 3539 1583 1770 3539 1583 1770 3539 1583 1570 3539 1583 1583 3539 1583 387 3539 1583 2583 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 387 3539 1583 383 3539 1583 383 3539 1583 383 3539 1583 383 3539 1583 383 3539 1583 383 3539 1583 383 383 3539
Satd. Flow (prot) 1787 1881 1599 3296 1742 0 1770 3539 1583 1770 3539 1583 Flt Permitted 0.608 0.950 0.048 0.208 0.208 Satd. Flow (perm) 1144 1881 1599 3296 1742 0 89 3539 1583 387 3539 1583 Right Turn on Red Yes Yes <td< td=""></td<>
Fit Permitted 0.608 0.950 0.048 0.208 Satd. Flow (perm) 1144 1881 1599 3296 1742 0 89 3539 1583 387 3539 1583 Right Turn on Red Yes Yes Yes Yes Yes 147 83 Link Speed (mph) 45 45 45 55 55 55 Link Speed (mph) 697 582 2066 1251 Travel Time (s) 10.6 8.8 25.6 15.5 Confl. Peds. (#hr) Peak Hour Factor 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86
Satd. Flow (perm) 1144 1881 1599 3296 1742 0 89 3539 1583 387 3539 1583 Right Turn on Red Yes Yes Yes Yes Yes Yes Yes Yes 83 Link Speed (mph) 45 45 55 55 55 55 55 55 155
Right Turn on Red Yes Yes Yes Yes Yes Yes Yes Seatcl. Flow (RTOR) 132 6 147 83 83 147 83 147 83 147 83 147 83 147 83 147 83 147 83 147 147 83 147 147 147 83 147 147 148
Said. Flow (RTOR) 132 6 147 83 Link Speed (mph) 45 45 55 55 55 Link Distance (ft) 697 582 2066 1251 15.5
Link Speed (mph) 45 45 55 55 Link Distance (ft) 697 582 2066 1251 Travel Time (s) 10.6 8.8 25.6 15.5 Confl. Peds. (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86
Link Distance (ft) 697 582 2066 1251 Travel Time (s) 10.6 8.8 25.6 15.5 Confl. Peds. (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86
Travel Time (s) 10.6 8.8 25.6 15.5 Confl. Peds. (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86
Confl. Peds. (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 0.86 0.8
Confl. Bikes (#/hr) Peak Hour Factor 0.86
Peak Hour Factor 0.86
Growth Factor 100% 100% 100% 100% 100% 100% 100% 100
Heavy venicies (%)
Parking (#/hr)
Parking (#/ni) Mid-Block Traffic (%) 0% 0% 0%
viid-Biock Fraffic (%) 0% 0% 0% Shared Lane Traffic (%)
· ,
Permitted Phases 8 4 6 2 2 6 Petroter Phases 7 4 5 3 9 5 5 3 3 1 4 7
Detector Phase 7 4 5 3 8 5 2 3 1 6 7
Switch Phase Minimum Initial (c) 70 70 70 70 70 70 70 70 70 70 70 70 70
Minimum Initial (s) 7.0 7.0 7.0 7.0 7.0 7.0 14.0 7.0 7.0 14.0 7.0 14.0 7.0
Winimum Split (s) 13.0 14.0 14.0 14.0 14.0 14.0 14.0 14.0 14.0 21.0 15.0 20.0 20.0 15.0 20.0
Total Split (s) 15.0 20.0 15.0 15.0 20.0 15.0 90.0 15.0 90.0 15.0 90.0 15.0 15.0 90.0 15.0 15.0 90.0 15.0 15.0 15.0 90.0 15.0 15.0 15.0 15.0 15.0 15.0 15.0 1
Total Split (%) 10.7% 14.3% 10.7% 10.7% 14.3% 10.7% 64.3% 10.7% 64.3% 10.7%
Yellow Time (s) 3.0 4.7 3.0 3.0 4.7 3.0 5.2 3.0 5.2 3.0
All-Red Time (s) 2.9 2.0 3.3 3.2 2.0 3.3 1.7 3.2 3.9 1.7 2.9
Lost Time Adjust (s) -0.9 -1.7 -1.3 -1.2 -1.7 -1.3 -1.9 -1.2 -1.9 -0.9
Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0
Lead/Lag Lead Lag Lag Lag Lag Lag Lead Lead Lead
Lead-Lag Optimize? Yes
Recall Mode None None None None None None C-Max None C-Max None
Act Effct Green (s) 25.0 15.1 25.1 9.9 15.5 95.0 88.8 103.8 96.0 85.0 94.5
Actuated g/C Ratio 0.18 0.11 0.18 0.07 0.11 0.68 0.63 0.74 0.69 0.61 0.68
//c Ratio 0.30 0.92 0.26 0.64 0.47 0.34 0.48 0.12 0.10 0.81 0.06
Control Delay 48.8 105.9 4.0 76.5 63.6 32.6 14.8 1.1 6.6 25.3 0.5
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.
Total Delay 48.8 105.9 4.0 76.5 63.6 32.6 14.8 1.1 6.6 25.3 0.5
LOS D F A E E C B A A C A
Approach Delay 65.4 71.5 14.1 24.1
Approach LOS´ E E B C
Oueue Length 50th (ft) 56 170 0 69 77 15 271 0 9 618 0
Queue Length 95th (ft) 98 #293 18 103 131 45 304 16 18 658 4
Internal Link Dist (ft) 617 502 1986 1171
Turn Bay Length (ft) 150 350 235 300 150 150 65
Base Capacity (vph) 254 202 394 235 198 180 2245 1211 366 2148 1100
Starvation Cap Reductr 0 0 0 0 0 0 0 0 0 0 0 0 0
Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0
Storage Cap Reductri 0 0 0 0 0 0 0 0 0 0 0 0
Reduced v/c Ratio 0.29 0.92 0.26 0.64 0.47 0.34 0.48 0.12 0.10 0.81 0.06
7.00 0.72 0.72 0.00 0.01 0.01 0.00 0.12 0.10 0.01 0.00

Area Type: Cycle Length: 140 Other

Actuated Cycle Length: 140
Offset: 119 (85%), Referenced to phase 2:NBSB and 6:NBSB, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 27.7 Intersection LOS: C
Intersection Capacity Utilization 70.7% ICU Level of Service C
Analysis Period (min) 15
Description: 05-1930
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: US 1 (Capital Blvd.) & Harris Road



	۶	•	₹I	•	†	L	ţ	1
Lane Group	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	W			ă	^	Ð	^	7
Traffic Volume (vph)	4	7	4	4	1103	4	1717	4
Future Volume (vph)	4	7	4	4	1103	4	1717	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12
Grade (%)	0%				0%		0%	
Storage Length (ft)	0	0		250		225		75
Storage Lanes	1	0		1		1		1
Taper Length (ft)	25			100		100		
Satd. Flow (prot)	1668	0	0	1770	3539	1770	3539	1583
Flt Permitted	0.984			0.950		0.950		
Satd. Flow (perm)	1668	0	0	1770	3539	1770	3539	1583
Link Speed (mph)	25				55		55	
Link Distance (ft)	716				691		2066	
Travel Time (s)	19.5				8.6		25.6	
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0
Parking (#/hr)								
Mid-Block Traffic (%)	0%				0%		0%	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	12	0	0	8	1226	4	1908	4
Sign Control	Stop				Free		Free	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 57.5%
Analysis Period (min) 15

ICU Level of Service B

-									_
Intersection									
Int Delay, s/veh	0.4								
Movement	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR	
Lane Configurations	Y	LDI	NDU	Ä	↑ ↑	1	*	7	
Traffic Vol, veh/h	4	7	4	4	1103	4	1717	4	
Future Vol, veh/h	4	7	4	4	1103	4	1717	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	- -	None	-	-	None	-	-	None	
Storage Length	0	-	-	250	-	225	-	75	
Veh in Median Storage, #	0	-		-	0	-	0	-	
Grade, %	0	-			0	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	
Mymt Flow	4	8	4	4	1226	4	1908	4	
		- 3	-	T	1220	,	1700		
Major/Minor	Minor2		Major1			Major2			
Conflicting Flow All	2548	954	1392	1908	0	894	-	0	
Stage 1	1917	-	-	-	-	-	-	-	
Stage 2	631	-	-	-	-	-	-	-	
Critical Hdwy	6.84	6.94	6.44	4.14	-	6.44	-	-	
Critical Hdwy Stg 1	5.84	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	5.84	-	-	-	-	-	-	-	
Follow-up Hdwy	3.52	3.32	2.52	2.22	-	2.52	-	-	
Pot Cap-1 Maneuver	22	259	185	307	-	388	-	-	
Stage 1	101	-	-	-	-	-	-	-	
Stage 2	492	-	-	-	-	-	-	-	
Platoon blocked, %					-		-	-	
Mov Cap-1 Maneuver	22	259	227	227	-	388	-	-	
Mov Cap-2 Maneuver	22	-	-	-	-	-	-	-	
Stage 1	101	-	-	-	-	-	-	-	
Stage 2	492	-	-	-	-	-	-	-	
Approach	EB		NB			SB			
HCM Control Delay, s	92.2		0.2			0			
HCM LOS	72.2 F		0.2			- 0			
110111 200	<u>'</u>								
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBU	SBT	SBR		
Capacity (veh/h)		227	-	53	388	-	-		
HCM Lane V/C Ratio		0.039	-	0.231	0.011	-	-		
HCM Control Delay (s)		0.039 21.5	-	92.2	14.4	-	-		
		0.039					-		

	۶	-	←	•	>	4	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	!
Lane Configurations	*	*	*	7		#	_
Traffic Volume (vph)	50	266	152	35	34	58	j
Future Volume (vph)	50	266	152	35	34	58	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	j
Lane Width (ft)	12	12	12	12	12	12	
Grade (%)		0%	0%		0%		
Storage Length (ft)	110			160	175	0	j
Storage Lanes	1			1	1	1	
Taper Length (ft)	100				100		
Satd. Flow (prot)	1770	1863	1863	1583	1770	1583	,
Flt Permitted	0.950				0.950		
Satd. Flow (perm)	1770	1863	1863	1583	1770	1583	í
Link Speed (mph)		45	45		25		
Link Distance (ft)		582	1003		715		
Travel Time (s)		8.8	15.2		19.5		
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.72	0.72	0.72	0.72	0.72	0.72	
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0)
Parking (#/hr)							
Mid-Block Traffic (%)		0%	0%		0%		
Shared Lane Traffic (%)							
Lane Group Flow (vph) Sign Control	69	369	211	49	47	81	
		Free	Free		Stop		

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 24.7%
Analysis Period (min) 15

ICU Level of Service A

-							
Intersection							
Int Delay, s/veh	2.5						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
	EDL	EDI		WDR	SDL 1	3DK	
Lane Configurations Traffic Vol., veh/h	1 50	T 266	↑ 152	r 35	1 34	r 58	
Future Vol, veh/h	50 50	266	152	35	34	58	
Conflicting Peds, #/hr	0	200	152	35 0	0	0	
Sign Control RT Channelized	Free	Free None	Free -	Free	Stop	Stop None	
	110	None -	-	None 160	- 17E	0	
Storage Length	110	0		100	175 0	0	
Veh in Median Storage, #			0				
Grade, %	-	0	0	- 70	0	-	
Peak Hour Factor	72	72	72	72	72	72	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	69	369	211	49	47	81	
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	211	0	-	0	719	211	
Stage 1	-	-	-	-	211	-	
Stage 2					508	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2					5.42	-	
Follow-up Hdwy	2.218				3.518	3.318	
Pot Cap-1 Maneuver	1360		-	-	395	829	
Stage 1	-				824	-	
Stage 2		_			604	-	
Platoon blocked, %							
Mov Cap-1 Maneuver	1360		-		375	829	
Mov Cap-2 Maneuver	-				375	-	
Stage 1			-		824		
Stage 2		-			573	_	
Olago 2					0,0		
			1.15				
Approach	EB		WB		SB		
HCM Control Delay, s	1.2		0		12.1		
HCM LOS					В		
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)		1360	-	-	-	375	829
HCM Lane V/C Ratio		0.051				0.126	0.097
HCM Control Delay (s)		7.8				16	9.8
HCM Lane LOS		7.0 A				C	7.0 A
HCM 95th %tile Q(veh)		0.2	_			0.4	0.3
HOW FOUT MINE Q(VEII)		0.2	-	-	-	0.4	0.3

	•	-	•	•	←	•	•	†	~	-	Ţ	4	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	*	†	7	77	ĵ»		*	^	7	ሻ	^	7	
Traffic Volume (vph)	100	104	107	148	113	28	98	1566	145	69	1457	44	
Future Volume (vph)	100	104	107	148	113	28	98	1566	145	69	1457	44	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
ane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12	
Grade (%)		-2%			8%			0%			0%		
Storage Length (ft)	150		350	235		0	300		150	150		65	
Storage Lanes	1		1	2		0	1		1	1		1	
Faper Length (ft)	100		•	200			100		•	100		•	
Satd. Flow (prot)	1787	1881	1599	3296	1735	0	1770	3539	1583	1770	3539	1583	
It Permitted	0.493	.001	.077	0.950	1700		0.062	0007	1000	0.061	0007	1000	
Satd. Flow (perm)	928	1881	1599	3296	1735	0	115	3539	1583	114	3539	1583	
Right Turn on Red	720	1001	Yes	3270	1755	Yes	113	3337	Yes	117	3337	Yes	
Satd. Flow (RTOR)			132		7	103			129			83	
, ,		45	132		45			55	127		55	03	
Link Speed (mph)		45 697			582						1251		
ink Distance (ft)								2066					
ravel Time (s)		10.6			8.8			25.6			15.5		
Confl. Peds. (#/hr)													
Confl. Bikes (#/hr)					2.5								
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	
Parking (#/hr)													
/lid-Block Traffic (%)		0%			0%			0%			0%		
Shared Lane Traffic (%)													
ane Group Flow (vph)	101	105	108	149	142	0	99	1582	146	70	1472	44	
Furn Type	D.P+P	NA	pm+ov	Prot	NA		D.P+P	NA	pm+ov	D.P+P	NA	pm+ov	
Protected Phases	7	4	5	3	8		5	2	3	1	6	7	
Permitted Phases	8		4				6		2	2		6	
Detector Phase	7	4	5	3	8		5	2	3	1	6	7	
Switch Phase													
Vinimum Initial (s)	7.0	7.0	7.0	7.0	7.0		7.0	14.0	7.0	7.0	14.0	7.0	
Vlinimum Split (s)	13.0	14.0	14.0	14.0	14.0		14.0	21.0	14.0	14.0	21.0	13.0	
Fotal Split (s)	30.0	25.0	25.0	20.0	15.0		25.0	75.0	20.0	20.0	70.0	30.0	
Total Split (%)	21.4%	17.9%	17.9%	14.3%	10.7%		17.9%	53.6%	14.3%	14.3%	50.0%	21.4%	
/ellow Time (s)	3.0	4.7	3.0	3.0	4.7		3.0	5.2	3.0	3.0	5.2	3.0	
All-Red Time (s)	2.9	2.0	3.3	3.2	2.0		3.3	1.7	3.2	3.9	1.7	2.9	
ost Time Adjust (s)	-0.9	-1.7	-1.3	-1.2	-1.7		-1.3	-1.9	-1.2	-1.9	-1.9	-0.9	
otal Lost Time (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	
_ead/Lag	Lead	Lag	Lag	Lead	Lag		Lag	Lag	Lead	Lead	Lead	Lead	
_ead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None		None	C-Max	None	None	C-Max	None	
Act Effct Green (s)	33.3	20.6	40.6	12.6	20.8		86.7	79.4	97.1	87.7	66.7	79.2	
Actuated g/C Ratio	0.24	0.15	0.29	0.09	0.15		0.62	0.57	0.69	0.63	0.48	0.57	
//c Ratio	0.34	0.38	0.19	0.50	0.54		0.32	0.79	0.13	0.37	0.87	0.05	
Control Delay	42.2	58.5	2.8	66.3	61.2		35.7	28.8	2.1	17.7	40.1	0.03	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.2	
Total Delay	42.2	58.5	2.8	66.3	61.2		35.7	28.8	2.1	17.7	40.1	0.2	
OS	42.2 D	50.5 E	2.0 A	00.3 E	01.2 E		33.7 D	20.0 C	Α.1	В	40.1 D	0.2 A	
Approach Delay	U	34.1	A	L	63.8		U	27.0	А	D	38.0	A	
Approach LOS		34.1 C			03.0 E			27.0 C			36.0 D		
	71	87	0	47			21		г	22		0	
Queue Length 50th (ft)	71		0	67	115		31	599	5	22	625	0	
Queue Length 95th (ft)	120	149	22	103	193		86	758	29	53	747	1	
nternal Link Dist (ft)		617	0==	-0	502			1986			1171		
Furn Bay Length (ft)	150		350	235	610		300	0000	150	150	4.0.	65	
Base Capacity (vph)	407	277	557	353	263		307	2008	1161	251	1686	1065	
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0	
	0	0	0	0	0		0	0	0	0	0	0	
Spillback Cap Reductn									0	0	0	^	
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	0 0.25	0 0.38	0 0.19	0 0.42	0 0.54		0.32	0 0.79	0 0.13	0 0.28	0 0.87	0 0.04	

Area Type: Other Cycle Length: 140

Actuated Cycle Length: 140
Offset: 25 (18%), Referenced to phase 2:NBSB and 6:NBSB, Start of Green

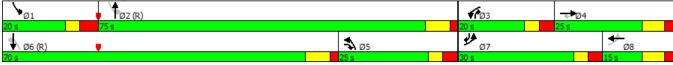
Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 34.6 Intersection LOS: C
Intersection Capacity Utilization 79.3% ICU Level of Service D
Analysis Period (min) 15
Description: 05-1930

Splits and Phases: 1: US 1 (Capital Blvd.) & Harris Road



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Lane Group	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	¥			ă	^	ū	^	7
Traffic Volume (vph)	4	11	4	11	1807	4	1711	4
Future Volume (vph)	4	11	4	11	1807	4	1711	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12
Grade (%)	0%				0%		0%	
Storage Length (ft)	0	0		250		225		75
Storage Lanes	1	0		1		1		1
Taper Length (ft)	25			100		100		
Satd. Flow (prot)	1655	0	0	1770	3539	1770	3539	1583
Flt Permitted	0.988			0.950		0.950		
Satd. Flow (perm)	1655	0	0	1770	3539	1770	3539	1583
Link Speed (mph)	25				55		55	
Link Distance (ft)	716				691		2066	
Travel Time (s)	19.5				8.6		25.6	
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0
Parking (#/hr)								
Mid-Block Traffic (%)	0%				0%		0%	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	16	0	0	16	2008	4	1901	4
Sign Control	Stop				Free		Free	
Interception Comments								

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 60.0%
Analysis Period (min) 15

-								
Intersection								
Int Delay, s/veh	0.8							
Movement	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	W			ă	^	Ð	^	7
Traffic Vol., veh/h	4	11	4	11	1807	4	1711	4
Future Vol, veh/h	4	11	4	11	1807	4	1711	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	·-	None	-	-	None	-	-	None
Storage Length	0	-		250	-	225		75
Veh in Median Storage, #	0	-	-	-	0	-	0	-
Grade, %	0	-	-	-	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2
Mvmt Flow	4	12	4	12	2008	4	1901	4
Major/Minor	Minor		Mojor1			Mojor?		
Major/Minor	Minor2	054	Major1	4004		Major2		
Conflicting Flow All	2947	951	1388	1901	0	1465	-	0
Stage 1	1910	-	-	-	-	-	-	-
Stage 2	1037	-	-	-	-	-	-	-
Critical Hdwy	6.84	6.94	6.44	4.14	-	6.44	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-	-	-
Critical Hdwy Stg 2	5.84				-		-	-
Follow-up Hdwy	3.52	3.32	2.52	2.22	-	2.52	-	-
Pot Cap-1 Maneuver	12	260	186	309	-	166	-	-
Stage 1	102	-	-	-	-	-	-	-
Stage 2	303	-	-	-	-	-	-	-
Platoon blocked, %					-		-	-
Mov Cap-1 Maneuver	12	260	258	258	-	166	-	-
Mov Cap-2 Maneuver	12	-	-	-	-	-	-	-
Stage 1	102	-	-	-	-	-	-	-
Stage 2	303	-	-	-	-	-	-	-
Approach	EB		NB			SB		
						0.1		
HCM Control Delay s	148.4		0.2					
HCM Control Delay, s	148.4		0.2			0.1		
HCM Control Delay, s HCM LOS	148.4 F		0.2			0.1		
HCM LOS				501.4	0011		000	
HCM LOS Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBU	SBT	SBR	
Minor Lane/Major Mvmt Capacity (veh/h)		258	NBT -	40	166	SBT -	-	
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio		258 0.065	NBT -	40 0.417	166 0.027	SBT - -		
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		258 0.065 19.9	NBT - -	40 0.417 148.4	166 0.027 27.3	SBT - -	-	
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio		258 0.065	NBT -	40 0.417	166 0.027	SBT - -	-	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 31.0%
Analysis Period (min) 15

-							
Intersection							
Int Delay, s/veh	4.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	147	100	101	7	\	11/	
Traffic Vol, veh/h	146	189	181	62	62	116	
Future Vol, veh/h	146	189	181	62	62	116	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None		None	
Storage Length	110	-	-	160	175	0	
Veh in Median Storage, #	-	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	172	222	213	73	73	136	
Major/Minor	Major1		Major?		Minor		
Major/Minor	Major1	^	Major2	^	Minor2	040	
Conflicting Flow All	213	0	-	0	779	213	
Stage 1	-	-	-	-	213	-	
Stage 2	-	-	-	-	566	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1357	-	-	-	364	827	
Stage 1	-	-	-	-	823	-	
Stage 2	-	-	-	-	568	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1357		-	-	318	827	
Mov Cap-2 Maneuver		-	-	-	318	-	
Stage 1	-	-	-	-	823	-	
Stage 2		-	-	-	496	-	
	ED		MP		0.5		
Approach	EB		WB		SB		
HCM Control Delay, s	3.5		0		13.5		
HCM LOS					В		
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR	SBLn1	SBLn2
		1357	EDI	WDI -	WDK -	318	827
Capacity (veh/h) HCM Lane V/C Ratio							
		0.127	-	-	-	0.229	0.165
HCM Control Delay (s)		8	-	-	-	19.7	10.2
HCM Lane LOS		Α	-	-	-	С	В
HCM 95th %tile Q(veh)		0.4	-	-	-	0.9	0.6

Appendix G:
Synchro Output:
Background (2022)

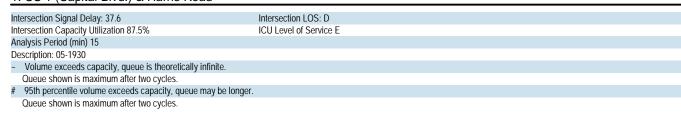
	•	→	•	•	+	•	•	†	<i>></i>	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	↑	7	ሻሻ	4		*	^	7	ሻ	^	7
Traffic Volume (vph)	72	180	99	149	76	16	93	1077	143	39	1799	63
Future Volume (vph)	72	180	99	149	76	16	93	1077	143	39	1799	63
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		-2%			8%			0%			0%	
Storage Length (ft)	150	270	350	235	070	0	300	070	150	150	070	65
Storage Lanes	1		1	2		0	1		1	1		1
Taper Length (ft)	100		•	200		Ū	100		•	100		•
Satd. Flow (prot)	1787	1881	1599	3296	1740	0	1770	3539	1583	1770	3539	1583
FIt Permitted	0.552	.001	1077	0.950	17.10	Ū	0.048	0007	1000	0.158	0007	1000
Satd. Flow (perm)	1039	1881	1599	3296	1740	0	89	3539	1583	294	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			132		6	. 00			159			83
Link Speed (mph)		45			45			55			55	
Link Distance (ft)		697			582			2066			1251	
Travel Time (s)		10.6			8.8			25.6			15.5	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)	, , , , , , , , , , , , , , , , , , ,	,								,		
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		3.3			3.0			0.0			0.0	
Lane Group Flow (vph)	84	209	115	173	107	0	108	1252	166	45	2092	73
Turn Type	D.P+P	NA	pm+ov	Prot	NA		D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	7	4	5	3	8		5	2	3	1	6	7
Permitted Phases	8		4				6		2	2		6
Detector Phase	7	4	5	3	8		5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0		7.0	14.0	7.0	7.0	14.0	7.0
Minimum Split (s)	13.0	14.0	14.0	14.0	14.0		14.0	21.0	14.0	14.0	21.0	13.0
Total Split (s)	15.0	20.0	15.0	15.0	20.0		15.0	90.0	15.0	15.0	90.0	15.0
Total Split (%)	10.7%	14.3%	10.7%	10.7%	14.3%		10.7%	64.3%	10.7%	10.7%	64.3%	10.7%
Yellow Time (s)	3.0	4.7	3.0	3.0	4.7		3.0	5.2	3.0	3.0	5.2	3.0
All-Red Time (s)	2.9	2.0	3.3	3.2	2.0		3.3	1.7	3.2	3.9	1.7	2.9
Lost Time Adjust (s)	-0.9	-1.7	-1.3	-1.2	-1.7		-1.3	-1.9	-1.2	-1.9	-1.9	-0.9
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag		Lag	Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None		None	C-Max	None	None	C-Max	None
Act Effct Green (s)	25.0	15.0	25.0	10.0	15.4		95.0	88.8	103.8	96.0	85.0	94.6
Actuated g/C Ratio	0.18	0.11	0.18	0.07	0.11		0.68	0.63	0.74	0.69	0.61	0.68
v/c Ratio	0.36	1.04	0.29	0.74	0.55		0.60	0.56	0.14	0.15	0.97	0.07
Control Delay	50.2	133.5	5.8	82.3	67.0		53.1	16.3	1.2	7.1	40.9	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.2	133.5	5.8	82.3	67.0		53.1	16.3	1.2	7.1	40.9	0.7
LOS	D	F	А	F	Е		D	В	Α	Α	D	А
Approach Delay		80.4			76.5			17.3			38.8	
Approach LOS		F			7 G.6			В			D	
Queue Length 50th (ft)	64	~205	0	80	89		43	340	2	11	917	0
Queue Length 95th (ft)	108	#346	27	#122	146		102	377	19	22	956	6
Internal Link Dist (ft)		617			502			1986			1171	
Turn Bay Length (ft)	150	0	350	235	302		300	.,	150	150		65
Base Capacity (vph)	241	201	393	235	196		180	2243	1214	308	2148	1100
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage oup reductii					U			0	U	U	U	U
Reduced v/c Ratio	0.35	1.04	0.29	0.74	0.55		0.60	0.56	0.14	0.15	0.97	0.07

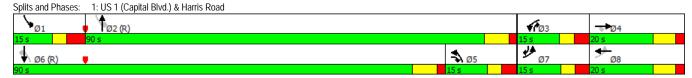
Area Type: Cycle Length: 140 Other

Actuated Cycle Length: 140
Offset: 119 (85%), Referenced to phase 2:NBSB and 6:NBSB, Start of Green

Natural Cycle: 120 Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.04





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Lane Group	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	W			ă	† †	Ð	† †	7
Traffic Volume (vph)	4	8	4	5	1311	4	2044	4
Future Volume (vph)	4	8	4	5	1311	4	2044	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12
Grade (%)	0%				0%		0%	
Storage Length (ft)	0	0		250		225		75
Storage Lanes	1	0		1		1		1
Taper Length (ft)	25			100		100		
Satd. Flow (prot)	1664	0	0	1770	3539	1770	3539	1583
Flt Permitted	0.985			0.950		0.950		
Satd. Flow (perm)	1664	0	0	1770	3539	1770	3539	1583
Link Speed (mph)	25				55		55	
Link Distance (ft)	716				691		2066	
Travel Time (s)	19.5				8.6		25.6	
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0
Parking (#/hr)								
Mid-Block Traffic (%)	0%				0%		0%	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	13	0	0	10	1457	4	2271	4
Sign Control	Stop				Free		Free	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 66.5%
Analysis Period (min) 15

Intersection								
Int Delay, s/veh	0.9							
Movement	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	Y EDL	EDK	NDU		<u>ND1</u>		<u>361</u>	3DK
Traffic Vol., veh/h	'T' 4	8	4	5	TT 1311	4	TT 2044	r 4
Future Vol, veh/h	4	8	4	5	1311	4	2044	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	Siup	None	riee	riee -	None	riee -	riee -	None
Storage Length	0	None -	-	250	None -	225	-	75
Veh in Median Storage, #	0		-	230	0	223	0	75
Grade, %	0	-	-	-	0		0	-
Peak Hour Factor	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	90	90	90	90	2	90	90
Mymt Flow	4	9	4	6	1457	4	2271	4
IVIVITIL FIOW	4	9	4	0	1437	4	2211	4
Major/Minor	Minor2		Major1			Major2		
Conflicting Flow All	3028	1136	1658	2271	0	1063	-	0
Stage 1	2280	-	-	-	-	-	-	-
Stage 2	748		-		-	-		
Critical Hdwy	6.84	6.94	6.44	4.14	-	6.44	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-		
Critical Hdwy Stg 2	5.84		-	-	-		-	-
Follow-up Hdwy	3.52	3.32	2.52	2.22	-	2.52		
Pot Cap-1 Maneuver	10	196	124	221	-	302	-	-
Stage 1	63	-	-	-		-		
Stage 2	429		-	-	-		-	-
Platoon blocked. %	,							
Mov Cap-1 Maneuver	10	196	160	160	-	302		-
Mov Cap-1 Maneuver	10	-	-	-	-	-		
Stage 1	63		-	-	-	-		-
Stage 2	429			-				
Stage 2	427			-			-	
Approach	EB		NB			SB		
HCM Control Delay, s	230.8		0.2			0		
HCM LOS	F							
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBU	SBT	SBR	
Capacity (veh/h)		160	IND I	27	302	301	JDK -	
HCM Lane V/C Ratio		0.063	-	0.494	0.015	-		
HCM Control Delay (s)		29	-	230.8	17.1	-	-	
, , ,		29 D	-	230.8 F	17.1 C		-	
HCM Lane LOS		0.2	-	1.5	0	-	-	
HCM 95th %tile Q(veh)		0.2	-	1.5	U	-	-	

	•	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	*	*	1	*	1
Traffic Volume (vph)	61	299	171	44	43	70
Future Volume (vph)	61	299	171	44	43	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	110			160	175	0
Storage Lanes	1			1	1	1
Taper Length (ft)	100				100	
Satd. Flow (prot)	1770	1863	1863	1583	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1770	1863	1863	1583	1770	1583
Link Speed (mph)		45	45		25	
Link Distance (ft)		582	1003		715	
Travel Time (s)		8.8	15.2		19.5	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.72	0.72	0.72	0.72	0.72	0.72
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	85	415	238	61	60	97
Sign Control		Free	Free		Stop	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 25.7%
Analysis Period (min) 15

Intersection							
Int Delay, s/veh	2.9						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	<u> </u>	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	7	30L	7	
Traffic Vol, veh/h	61	299	171	44	43	70	
Future Vol, veh/h	61	299	171	44	43	70	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	110	-	-	160	175	0	
Veh in Median Storage, #	-	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	72	72	72	72	72	72	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	85	415	238	61	60	97	
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	238	0		0	823	238	
Stage 1	-	-	-	-	238	-	
Stage 2		-	-	-	585		
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1329	-	-	-	343	801	
Stage 1	-	-	-	-	802	-	
Stage 2	-	-	-	-	557	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1329	-	-	-	321	801	
Mov Cap-2 Maneuver	-	-	-	-	321	-	
Stage 1	-	-	-	-	802	-	
Stage 2	-	-	-	-	521	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.3		0		13.4		
HCM LOS					В		
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)		1329	EDI	WDI	WDK	321	801
HCM Lane V/C Ratio		0.064	-	-		0.186	0.121
HCM Control Delay (s)		7.9				18.8	10.1
HCM Lane LOS		Α.,	-	-	-	10.0 C	В
HCM 95th %tile Q(veh)		0.2				0.7	0.4
TOWN 70th 70the Q(Veh)		0.2				0.7	0.4

rowth Factor 100% 100% 100% 100% 100% 100% 100% 100		٠	→	•	•	+	•	•	†	/	/	↓	4
ane Configurations and Configuration and Configurations and Configurations and Configurations and Configuration and Configuration and Configuration and Configuration	Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
raffic Volume (uph) 113 120 120 176 129 32 227 1889 166 90 1707 5.0 scal Flow (phply) 1900 1900 1900 1900 1900 1900 1900 190													
ulture Volume (volph) 131 120 120 176 129 32 227 1889 166 90 170 750 190 1900							32						
Real Flow (phip) 1900 19													
Common C													
Table Tabl													
Introspic Length (ft) 150 350 235 0 300 150 1	. ,	12		12	12		12	12		12	12		
Incarge Lanes 1	` '	150	270	350	235	070	0	300	070	150	150	070	65
Separation 100	0 0 17												
aids Flow (prop) 1787 1881 1599 3296 1735 0 1770 2339 1583 1770 3539 1583 1891 1770 3539 1583 1770 3539 1770 3530 1770	•			•			U			•			•
Permitted			1881	1599		1735	0		3539	1583		3539	1583
alds. Flow (perm) (perm			.00.	1077		1700	Ū		0007	1000		0007	.000
Septemborne			1881	1599		1735	0		3539	1583		3539	1583
gaint Flow (RTOR) 45 32 7 129 129 83 ink Distance (I) 697 582 2066 1251 155 ravel Time (s) 10.6 8.8 25.6 15.5 15.5 ontl. Peds. (#m) 0011. Beles (#m) 8.8 25.6 0.9 15.5 ontl. Reds. (#m) 0011. Beles (#m) 009 0.99	4 /	007	.00.		0270	1700			0007			0007	
ink Spead (mph)	0					7							
ink Distance (ft)	, ,		45	.02					55	,		55	00
ravel Time (s)													
onli. Pieks, (#hr) onli. Bikes	. ,												
onth. Bites (#hr) eack Hour Factor 0.99 0.99 0.99 0.99 0.99 0.99 0.99 0.9	. , ,		.0.0			0.0			20.0			70.0	
eak Hour Factor	• •												
rowth Factor 100% 1	Peak Hour Factor	0 99	0 99	0 99	0 99	0 99	0 99	0 99	0 99	0 99	0 99	0 99	0 99
leary Vehicles (%)	Growth Factor												
us Blockages (#hr) arking (#hr) the Block Traffic (%) bared Lane Traffic (%) ane Group Flow (vph) 114 121 121 178 162 0 229 1908 168 91 1724 51 1707 1708 1709 1714 1714 171 171 171 171 171													
arking (#m) ilid-Block Traffic (%) anne Group Flow (vph) In 14	, ,												
International Profession 1	J (/	J	3	J	, , , , , , , , , , , , , , , , , , ,	<u> </u>	J	J	J	3	3	3	3
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ame Group Flow (vph)	. ,		0,0			070			070			070	
turn Type D.P+P NA pm+ov Prot NA D.P+P NA pm+ov D.P+P NA pm+ov orlocted Phases 7 4 5 3 8 5 2 3 1 6 7 erelector Phase 7 4 5 3 8 5 2 3 1 6 7 witch Phase 1 4 5 3 8 5 2 3 1 6 7 linimum Initial (s) 7.0 7.0 7.0 7.0 7.0 7.0 14.0 14.0 21.0 14.0 14.0 21.0 13.0 13.0 25.0 25.0 20.0 15.0 25.0 20.0 20.0 20.0 30.0 25.0 25.0 20.0 15.0 25.0 20.0 20.0 30.0 22.1 33.0 3.1 17.2 29.0 33.0 3.2 20.0 33.3 1.7 3.2 3.9 1.7 2	, ,	114	121	121	178	162	0	229	1908	168	91	1724	51
rotected Phases 7 4 5 3 8 5 2 3 1 6 7 emitted Phases 8 4 4 6 6 2 2 2 6 6 elector Phases 7 4 5 3 8 5 2 3 1 6 7 witch Phase 8 7 4 5 3 8 5 2 3 1 6 6 7 7 witch Phase 8 7 4 5 3 8 5 2 3 1 6 6 7 7 witch Phase 8 7 7 4 5 5 3 8 5 5 2 3 1 1 6 7 7 witch Phase 8 7 7 4 5 5 3 8 5 5 2 3 1 1 6 7 7 witch Phase 8 7 7 7 7 7 7 7 7 7 7 7 7 1 1 1 1 1 1 1							J						
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otal Split (s) 30.0 25.0 25.0 20.0 15.0 25.0 75.0 20.0 20.0 70.0 30.0 otal Split (%) 21.4% 17.9% 17.9% 14.3% 10.7% 17.9% 53.6% 14.3% 14.3% 50.0% 21.4% ellow Time (s) 3.0 4.7 3.0 3.0 4.7 3.0 5.2 3.0 3.0 5.2 3.0 li-Red Time (s) 2.9 2.0 3.3 3.2 2.0 3.3 1.7 3.2 3.9 1.7 2.9 ost Time Adjust (s) -0.9 -1.7 -1.3 -1.2 -1.7 -1.3 -1.9 -1.9 -0.9 -0.9 -0.1 3.1 2 -1.7 -1.3 -1.9 -1.9 -0.9 -0.9 -0.5 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0													
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II-Red Time (s) 2.9 2.0 3.3 3.2 2.0 3.3 1.7 3.2 3.9 1.7 2.9 ost Time Adjust (s) -0.9 -1.7 -1.3 -1.2 -1.7 -1.3 -1.9 -1.2 -1.9 -1.9 -0.9 otal Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 caal/Lag Lead Lag Lag Lad Lag Lag Lag Lag Lag Lag Lad Lead Lead caad-Lag Optimize? Yes cacal IMode None None None None None None None C-Max None ct Effet Green (s) 35.0 21.6 41.6 13.4 21.7 85.0 73.9 92.3 85.0 65.0 78.3 ctuated g/C Ratio 0.25 0.15 0.30 0.10 0.16 0.61 0.53 0.66 0.61 0.46 0.56 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.59 0.75 1.02 0.15 0.47 1.05 0.66 c Ratio 0.38 0.42 0.21 0.57 0.59 0.59 0.75 1.02 0.15 0.47 0.06 c Ratio 0.50 0.50 0.50 0.50 0.50 0.50 0													
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ct Effct Green (s) 35.0 21.6 41.6 13.4 21.7 85.0 73.9 92.3 85.0 65.0 78.3 ctuated g/C Ratio 0.25 0.15 0.30 0.10 0.16 0.61 0.53 0.66 0.61 0.46 0.56 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.06 ontrol Delay 42.4 59.2 3.9 67.5 63.0 62.7 59.4 2.9 27.2 73.0 0.4 usue Delay 0.0	Recall Mode												
ctuated g/C Ratio 0.25 0.15 0.30 0.10 0.16 0.61 0.53 0.66 0.61 0.46 0.56 c Ratio 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.06 control Delay 42.4 59.2 3.9 67.5 63.0 62.7 59.4 2.9 27.2 73.0 0.4 drueue Delay 0.0													
Recomposition 0.38 0.42 0.21 0.57 0.59 0.75 1.02 0.15 0.47 1.05 0.06 control Delay 42.4 59.2 3.9 67.5 63.0 62.7 59.4 2.9 27.2 73.0 0.4 drueue Delay 0.0 </td <td>• • •</td> <td></td>	• • •												
ontrol Delay													
Aueue Delay 0.0 <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>													
otal Delay 42.4 59.2 3.9 67.5 63.0 62.7 59.4 2.9 27.2 73.0 0.4 OS D E A E E E E E A C E A pproach Delay 35.0 65.3 55.6 68.8 E<													
OS D E A E E E E E A C E A pproach Delay 35.0 65.3 55.6 68.8 pproach LOS D E A 25 25	,												
pproach Delay pproach LOS 35.0 65.3 55.6 68.8 pproach LOS D E E E E useue Length 50th (ft) 80 102 0 80 133 149 ~914 11 29 ~897 0 useue Length 95th (ft) 133 169 30 120 #243 #264 #1153 39 86 #1035 3 sternal Link Dist (ft) 617 502 1986 1171	,												
Proposed LOS		<i>D</i>						L		A			
Leueue Length 50th (ft) 80 102 0 80 133 149 ~914 11 29 ~897 0 tueue Length 95th (ft) 133 169 30 120 #243 #264 #1153 39 86 #1035 3 sternal Link Dist (ft) 617 502 1986 1171 turn Bay Length (ft) 150 350 235 300 150 150 65 ase Capacity (vph) 408 289 567 353 275 306 1867 1103 242 1643 1047 tarvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0													
Authorized Reduction 133 169 30 120 #243 #264 #1153 39 86 #1035 3 Internal Link Dist (ft) 617 502 1986 1171 1171 urn Bay Length (ft) 150 350 235 300 150 150 65 ase Capacity (vph) 408 289 567 353 275 306 1867 1103 242 1643 1047 tarvation Cap Reductn 0		RΩ		n	80			1/10		11	20		n
Internal Link Dist (ft) 617 502 1986 1171 urn Bay Length (ft) 150 350 235 300 150 150 65 ase Capacity (vph) 408 289 567 353 275 306 1867 1103 242 1643 1047 tarvation Cap Reductn 0 <													
urn Bay Length (ft) 150 350 235 300 150 150 65 ase Capacity (vph) 408 289 567 353 275 306 1867 1103 242 1643 1047 tarvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 pillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 torage Cap Reductn 0 0 0 0 0 0 0 0 0		133		30	120			# ZU4		- 3)	- 00		J
ase Capacity (vph) 408 289 567 353 275 306 1867 1103 242 1643 1047 tarvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 pillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 torage Cap Reductn 0 0 0 0 0 0 0 0 0		150	017	350	225	302		300	1 700	150	150	1171	45
tarvation Cap Reductn 0			280			275			1947			16/12	
pillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 torage Cap Reductn 0 0 0 0 0 0 0 0 0 0 0													
torage Cap Reductn 0 0 0 0 0 0 0 0 0 0 0													
educed we remo 0.20 0.42 0.21 0.30 0.37 0.73 1.02 0.13 0.36 1.05 0.05													
	ACUULCU WE KAIIU	0.26	0.42	U.Z I	0.50	0.59		0.75	1.02	0.13	0.36	1.03	0.05

Area Type: Cycle Length: 140 Other

Actuated Cycle Length: 140
Offset: 25 (18%), Referenced to phase 2:NBSB and 6:NBSB, Start of Green

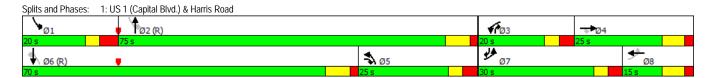
Natural Cycle: 100 Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.05

Intersection Signal Delay: 59.8 Intersection LOS: E
Intersection Capacity Utilization 91.4% ICU Level of Service F
Analysis Period (min) 15
Description: 05-1930

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



	•	•	₹î	1	†	L	↓	1
Lane Group	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	¥			ă	^	Ð	† †	7
Traffic Volume (vph)	4	12	4	12	2280	4	2002	4
Future Volume (vph)	4	12	4	12	2280	4	2002	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12
Grade (%)	0%				0%		0%	
Storage Length (ft)	0	0		250		225		75
Storage Lanes	1	0		1		1		1
Taper Length (ft)	25			100		100		
Satd. Flow (prot)	1651	0	0	1770	3539	1770	3539	1583
Flt Permitted	0.988			0.950		0.950		
Satd. Flow (perm)	1651	0	0	1770	3539	1770	3539	1583
Link Speed (mph)	25				55		55	
Link Distance (ft)	716				691		2066	
Travel Time (s)	19.5				8.6		25.6	
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0
Parking (#/hr)								
Mid-Block Traffic (%)	0%				0%		0%	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	17	0	0	17	2533	4	2224	4
Sign Control	Stop				Free		Free	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 73.0%
Analysis Period (min) 15

Intersection										
Int Delay, s/veh	2.5									
Movement	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR		
Lane Configurations	Y			ă	† †	Ð	† †	7		_
Traffic Vol, veh/h	4	12	4	12	2280	4	2002	4		
Future Vol, veh/h	4	12	4	12	2280	4	2002	4		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	-	None	-	-	None	-	-	None		
Storage Length	0	-	-	250	-	225	-	75		
Veh in Median Storage, #	0	-	-	-	0	-	0	-		
Grade, %	0	-	-		0	-	0	-		
Peak Hour Factor	90	90	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2	2	2		
Mvmt Flow	4	13	4	13	2533	4	2224	4		
Major/Minor	Minor2		Major1			Major2				
Conflicting Flow All	3535	1112	1624	2224	0	1849	_	0		
Stage 1	2233	1112	1024	2224	-	1049	-	-		
Stage 2	1302			-			-	-		
Critical Hdwy	6.84	6.94	6.44	4.14	-	6.44	-	-		
Critical Hdwy Stg 1	5.84	0.94	0.44	4.14	-	0.44	-	-		
Critical Hdwy Stg 2	5.84	-		-	-	-		-		
	3.52	3.32	2.52	2.22	-	2.52		-		
Follow-up Hdwy Pot Cap-1 Maneuver	3.52 ~ 4	203	131	2.22	-	93	-	-		
	~ 4 67	203	131	231	-	93	-	-		
Stage 1 Stage 2	219	-	-	-	-	-	-	-		
Platoon blocked, %	219	-	-	-	-	-	-	-		
Mov Cap-1 Maneuver	~ 4	203	189	189	-	93	-	-		
	~ 4	203	109	109	-	93	-	-		
Mov Cap-2 Maneuver Stage 1	~ 4 67	-	-		-	-	-	-		
	219		-	-	-		-	-		
Stage 2	219	-	-	-	-	-	-	-		
Approach	EB		NB			SB				
HCM Control Delay, s	\$ 646.9		0.2			0.1				
HCM LOS	F									
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBU	SBT	SBR			
Capacity (veh/h)		189	INDI	15	93	<u> </u>	JDIK			
HCM Lane V/C Ratio		0.094	-	1.185	0.048		-			
HCM Control Delay (s)		26	-		45.6					
HCM Lane LOS		20 D	-	\$ 040.7 F	45.0 E					
HCM 95th %tile Q(veh)		0.3	-	2.8	0.1	-	-			
		0.5		2.0	0.1		-			
Notes										
~: Volume exceeds capacity	\$: Delay	exceeds	300s +	: Computa	tion Not D	efined	*: All majo	r volume	in platoon	

	٠	-	•	•	>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	A	*	#	*	#
Traffic Volume (vph)	182	213	204	88	84	143
Future Volume (vph)	182	213	204	88	84	143
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	110			160	175	0
Storage Lanes	1			1	1	1
Taper Length (ft)	100				100	
Satd. Flow (prot)	1770	1863	1863	1583	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1770	1863	1863	1583	1770	1583
Link Speed (mph)		45	45		25	
Link Distance (ft)		582	1003		715	
Travel Time (s)		8.8	15.2		19.5	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	214	251	240	104	99	168
Sign Control		Free	Free		Stop	
Intersection Summary						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 35.5%
Analysis Period (min) 15

•							
Intersection							
Int Delay, s/veh	5.9						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
	EDL	<u>EDI</u>		WDK	SDL N	SDR 7	
Lane Configurations Traffic Vol., veh/h	182	T 213	↑ 204	88 L	"1 84	143	
Future Vol, veh/h	182	213	204	88	84	143	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	310p -	None	
Storage Length	110	-		160	175	0	
Veh in Median Storage, #	-	0	0	-	0	-	
Grade, %		0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	214	251	240	104	99	168	
IVIVIIIL I IUW	214	231	240	104	77	100	
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	240	0	-	0	919	240	
Stage 1	-	-	-	-	240	-	
Stage 2	-	-	-	-	679	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1327	-	-	-	301	799	
Stage 1	-	-	-	-	800	-	
Stage 2	-	-	-	-	504	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1327	-	-	-	252	799	
Mov Cap-2 Maneuver	-	-	-	-	252	-	
Stage 1	-	-	-	-	800	-	
Stage 2	-	-	-	-	423	-	
Approach	EB		WB		SB		
HCM Control Delay, s	3.8		0		17.2		
HCM LOS	3.0		U		17.2 C		
TICIVI LUS					C		
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)		1327	-	-	-	252	799
HCM Lane V/C Ratio		0.161	-	-	-	0.392	0.211
HCM Control Delay (s)		8.2	-	-	-	28.2	10.7
HCM Lane LOS		Α	-	-	-	D	В
HCM 95th %tile Q(veh)		0.6	-	-	-	1.8	8.0

Appendix H:

Synchro Output:

Build-out (2022)

	۶	→	•	•	←	•	₹I	4	†	/	\	ļ	4	
ane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	*	†	1	ሻሻ	1>			ă	^	7	ሻ	^	#	
Fraffic Volume (vph)	72	180	99	189	76	16	34	93	1094	143	41	1803	63	
uture Volume (vph)	72	180	99	189	76	16	34	93	1094	143	41	1803	63	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
1117	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	
ane Width (ft)	12		12	12		12	12	12		12	12		12	
Grade (%)	450	-2%	050		8%				0%	450	450	0%		
Storage Length (ft)	150		350	235		0		400		150	150		65	
Storage Lanes	1		1	2		0		1		1	1		1	
Taper Length (ft)	100			200				100			100			
Satd. Flow (prot)	1787	1881	1599	3296	1740	0	0	1770	3539	1583	1770	3539	1583	
It Permitted	0.552			0.950				0.048			0.153			
Satd. Flow (perm)	1039	1881	1599	3296	1740	0	0	89	3539	1583	285	3539	1583	
Right Turn on Red			Yes			Yes				Yes			Yes	
Satd. Flow (RTOR)			132		6	. 00				156			83	
ink Speed (mph)		45	102		45				55	100		55	03	
ink Distance (ft)		697			582				1860			1251		
ravel Time (s)		10.6			8.8				23.1			15.5		
Confl. Peds. (#/hr)														
Confl. Bikes (#/hr)														
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.90	0.86	0.86	0.86	0.86	0.86	0.86	
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	0	
Parking (#/hr)	3	J	J	J	J	J	J	J	0	0	J	J	U	
Aid-Block Traffic (%)		0%			0%				0%			0%		
. ,		0%			U70				U70			U76		
Shared Lane Traffic (%)					407				4070					
ane Group Flow (vph)	84	209	115	220	107	0	0	146	1272	166	48	2097	73	
Furn Type	D.P+P	NA	pm+ov	Prot	NA		D.P+P	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov	
Protected Phases	7	4	5!	3	8		5!	5	2	3	1	6	7	
Permitted Phases	8		4				6	6		2	2		6	
Detector Phase	7	4	5	3	8		5	5	2	3	1	6	7	
Switch Phase														
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0		7.0	7.0	14.0	7.0	7.0	14.0	7.0	
Viinimum Split (s)	13.0	14.0	14.0	14.0	14.0		14.0	14.0	21.0	14.0	14.0	21.0	13.0	
Fotal Split (s)	15.0	20.0	15.0	15.0	20.0		15.0	15.0	90.0	15.0	15.0	90.0	15.0	
Fotal Split (%)	10.7%	14.3%	10.7%	10.7%	14.3%		10.7%	10.7%	64.3%	10.7%	10.7%	64.3%	10.7%	
'ellow Time (s)	3.0	4.7	3.0	3.0	4.7		3.0	3.0	5.2	3.0	3.0	5.2	3.0	
All-Red Time (s)	2.9	2.0	3.3	3.2	2.0		3.3	3.3	1.7	3.2	3.9	1.7	2.9	
ost Time Adjust (s)	-0.9	-1.7	-1.3	-1.2	-1.7			-1.3	-1.9	-1.2	-1.9	-1.9	-0.9	
otal Lost Time (s)	5.0	5.0	5.0	5.0	5.0			5.0	5.0	5.0	5.0	5.0	5.0	
.ead/Lag	Lead	Lag	Lag	Lead	Lag		Lag	Lag	Lag	Lead	Lead	Lead	Lead	
.ead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None		None	None	C-Max	None	None	C-Max	None	
							INOHE							
Act Effet Green (s)	25.0	15.0	25.0	10.0	15.4			95.0	88.7	103.7	96.0	85.0	94.6	
Actuated g/C Ratio	0.18	0.11	0.18	0.07	0.11			0.68	0.63	0.74	0.69	0.61	0.68	
/c Ratio	0.36	1.04	0.29	0.94	0.55			0.81	0.57	0.14	0.16	0.98	0.07	
Control Delay	50.2	133.5	5.8	108.2	67.0			76.2	16.5	1.3	7.3	41.3	0.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0	0.0	0.0	0.0	0.0	
otal Delay	50.2	133.5	5.8	108.2	67.0			76.2	16.5	1.3	7.3	41.3	0.7	
.OS	D	F	Α	F	Е			Е	В	Α	Α	D	Α	
Approach Delay		80.4		•	94.7				20.4	•		39.3		
approach LOS		F			74.7 F				20.4 C			37.3 D		
Queue Length 50th (ft)	64	~205	0	104	89			80		2	12	922	0	
									347					
Queue Length 95th (ft)	108	#346	27	#173	146			#178	387	20	23	961	6	
nternal Link Dist (ft)		617			502				1780			1171		
urn Bay Length (ft)	150		350	235				400		150	150		65	
Base Capacity (vph)	241	201	393	235	196			180	2243	1213	302	2148	1100	
Starvation Cap Reductn	0	0	0	0	0			0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0			0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0			0	0	0	0	0	0	
storage oup redudent					0.55			0.81	0.57	0.14	0.16	0.98	0.07	
Reduced v/c Ratio	0.35	1.04	0.29	0.94	() 55									

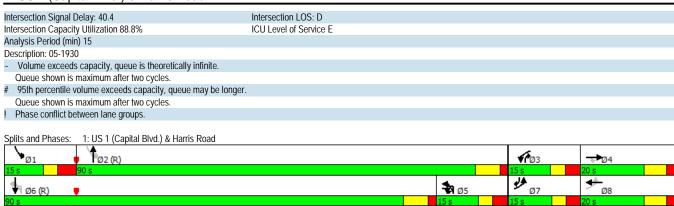
Area Type: Cycle Length: 140 Other

Actuated Cycle Length: 140
Offset: 119 (85%), Referenced to phase 2:NBSB and 6:NBSB, Start of Green

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.04



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Lane Group	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	¥			ă	† †	Ð	† †	7
Traffic Volume (vph)	4	8	4	5	1336	4	2118	4
Future Volume (vph)	4	8	4	5	1336	4	2118	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12
Grade (%)	0%				0%		0%	
Storage Length (ft)	0	0		250		225		75
Storage Lanes	1	0		1		1		1
Taper Length (ft)	25			100		100		
Satd. Flow (prot)	1664	0	0	1770	3539	1770	3539	1583
Flt Permitted	0.985			0.950		0.950		
Satd. Flow (perm)	1664	0	0	1770	3539	1770	3539	1583
Link Speed (mph)	25				55		55	
Link Distance (ft)	716				691		207	
Travel Time (s)	19.5				8.6		2.6	
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0
Parking (#/hr)								
Mid-Block Traffic (%)	0%				0%		0%	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	13	0	0	10	1484	4	2353	4
Sign Control	Stop				Free		Free	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 68.5%
Analysis Period (min) 15

Intersection								
Int Delay, s/veh	1							
Movement	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	Y EBL	LDK	NDU		† †		<u>≯</u>	JDK 7
		0	4	À		ð		
Traffic Vol, veh/h Future Vol, veh/h	4	8	4	5	1336 1336	4	2118 2118	4
	0	8	4	5 0	1336	4	2118	0
Conflicting Peds, #/hr								
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	None	-	-	None	-	-	None
Storage Length	0	-	-	250	-	225	-	75
Veh in Median Storage, #	0	-	-	-	0	-	0	-
Grade, %	0	-	-	-	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2
Mvmt Flow	4	9	4	6	1484	4	2353	4
Major/Minor	Minor2		Major1			Major2		
	3124	1177	1718	2353	0	1083		0
Conflicting Flow All	2362		1/18	2353		1083	-	
Stage 1		-			-		-	-
Stage 2	762	-	-	-	-	- (44	-	-
Critical Hdwy	6.84	6.94	6.44	4.14	-	6.44	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-	-	-
Critical Hdwy Stg 2	5.84				-		-	-
Follow-up Hdwy	3.52	3.32	2.52	2.22	-	2.52	-	-
Pot Cap-1 Maneuver	9	184	114	205	-	294	-	-
Stage 1	57	-	-	-	-	-	-	-
Stage 2	421	-	-	-	-	-	-	-
Platoon blocked, %					-		-	-
Mov Cap-1 Maneuver	9	184	147	147	-	294	-	-
Mov Cap-2 Maneuver	9	-	-	-	-	-	-	-
Stage 1	57	-		-	-	-	-	-
Stage 2	421	-	-	-	-	-	-	-
A	F2		ND			CD		
Approach	EB		NB			SB		
HCM Control Delay, s	257.5		0.2			0		
HCM LOS	F							
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBU	SBT	SBR	
Capacity (veh/h)		147	-	25	294	- 301	JUIX -	
HCM Lane V/C Ratio		0.068	-	0.533	0.015	-	-	
HCM Control Delay (s)		0.000	-	0.555				
DU WE ONIO DEIN IS		21.2		257.5	17 /			
		31.2	-	257.5	17.4	-	-	
HCM Lane LOS HCM 95th %tile Q(veh)		31.2 D 0.2	-	257.5 F 1.6	17.4 C 0		-	

	۶	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	*	*	#	*	1
Traffic Volume (vph)	61	301	211	44	43	70
Future Volume (vph)	61	301	211	44	43	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	110			160	175	0
Storage Lanes	1			1	1	1
Taper Length (ft)	100				100	
Satd. Flow (prot)	1770	1863	1863	1583	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1770	1863	1863	1583	1770	1583
Link Speed (mph)		45	45		25	
Link Distance (ft)		582	1249		715	
Travel Time (s)		8.8	18.9		19.5	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.72	0.72	0.72	0.72	0.72	0.72
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	85	418	293	61	60	97
Sign Control		Free	Free		Stop	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 27.8%
Analysis Period (min) 15

Intersection Int Delay, s/veh 2.9
Int Delay, s/veh 2.9
Movement EBL EBT WBT WBR SBL SBR
Lane Configurations 1 1 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
Traffic Vol, veh/h 61 301 211 44 43 70
Future Vol, veh/h 61 301 211 44 43 70
Conflicting Peds, #/hr 0 0 0 0 0 0
Sign Control Free Free Free Stop Stop
RT Channelized - None - None - None
Storage Length 110 160 175 0
Veh in Median Storage, # - 0 0 - 0 -
Grade, % - 0 0 - 0 -
Peak Hour Factor 72 72 72 72 72 72
Heavy Vehicles, % 2 2 2 2 2 2
Mvmt Flow 85 418 293 61 60 97
Major/Minor Major1 Major2 Minor2
Conflicting Flow All 293 0 - 0 881 293
Stage 1 293 -
Stage 2 588 -
Critical Hdwy 4.12 6.42 6.22
Critical Hdwy Stg 1 5.42 - 5.42
Critical Hdwy Stg 2 5.42 -
Follow-up Hdwy 2.218 3.518 3.318
Pot Cap-1 Maneuver 1269 317 746
Stage 1 757 -
Stage 2 555 -
Platoon blocked, % 555 -
•
Mov Can-2 Maneuver 296 -
Stage 1 757 -
Stage 1 757 -
Stage 1 757 -
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB HCM Control Delay, s 1.4 0 14.2
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB HCM Control Delay, s 1.4 0 14.2 HCM LOS B
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB HCM Control Delay, s 1.4 0 14.2 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB HCM Control Delay, s 1.4 0 14.2 HCM LOS B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) 1269 - - 296 746
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB - HCM Control Delay, s 1.4 0 14.2 - HCM LOS B B B -
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB - HCM Control Delay, s 1.4 0 14.2 HCM LOS B B B Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 SBLn2 Capacity (veh/h) 1269 - - 296 746 HCM Lane V/C Ratio 0.067 - - 0.202 0.13 HCM Control Delay (s) 8 - - 20.2 10.5
Stage 1 - - - 757 - Stage 2 - - - 518 - Approach EB WB SB - HCM Control Delay, s 1.4 0 14.2 - HCM LOS B B B -

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	?
Lane Configurations	1			4	¥		
Traffic Volume (vph)	343	4	7	215	40	23	3
Future Volume (vph)	343	4	7	215	40	23	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900)
Lane Width (ft)	12	12	12	12	12	12)
Grade (%)	0%			0%	0%		
Storage Length (ft)		0	0		0	0)
Storage Lanes		0	0		1	0)
Taper Length (ft)			100		25		
Satd. Flow (prot)	1861	0	0	1859	1717	0)
Flt Permitted				0.998	0.970		
Satd. Flow (perm)	1861	0	0	1859	1717	0)
Link Speed (mph)	45			45	25		
Link Distance (ft)	1249			503	515		
Travel Time (s)	18.9			7.6	14.0		
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90)
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	5
Bus Blockages (#/hr)	0	0	0	0	0	0)
Parking (#/hr)							
Mid-Block Traffic (%)	0%			0%	0%		
Shared Lane Traffic (%)							
Lane Group Flow (vph)	385	0	0	247	70	0)
Sign Control	Free			Free	Stop		

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 28.6%
Analysis Period (min) 15

Intersection						
Int Delay, s/veh	1.4					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		_	र्स	¥	
Traffic Vol, veh/h	343	4	7	215	40	23
Future Vol, veh/h	343	4	7	215	40	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	381	4	8	239	44	26
			_			
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	386	0	637	383
Stage 1	-	-	-	-	383	-
Stage 2	-	-	-	-	254	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2		-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1172	-	441	664
Stage 1	-				689	
Stage 2			-	_	788	
Platoon blocked, %					700	
Mov Cap-1 Maneuver			1172		437	664
Mov Cap-1 Maneuver			- 1172		437	-
Stage 1		_			689	_
Ü	-	-	-		782	-
Stage 2	-	-	-	-	782	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		13.4	
HCM LOS					В	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		499	-	-	1172	-
HCM Lane V/C Ratio		0.14	-	-	0.007	-
HCM Control Delay (s)		13.4	-	-	8.1	0
HCM Lane LOS		B 0.5	-	-	Α	Α

	€	•	†	-	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	† †	7		^
Traffic Volume (vph)	0	51	1313	29	0	2125
Future Volume (vph)	0	51	1313	29	0	2125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%		0%			0%
Storage Length (ft)	0	0		150	0	
Storage Lanes	0	1		1	0	
Taper Length (ft)	25				25	
Satd. Flow (prot)	0	1611	3539	1583	0	3539
Flt Permitted						
Satd. Flow (perm)	0	1611	3539	1583	0	3539
Link Speed (mph)	25		55			55
Link Distance (ft)	823		207			1860
Travel Time (s)	22.4		2.6			23.1
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	57	1459	32	0	2361
Sign Control	Stop		Free			Free
Intersection Summary						
Aroa Tuno:	Othor					

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 62.1%
Analysis Period (min) 15

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	WDR	<u>ND1</u>	NDK 7	SDL	<u>361</u>
Traffic Vol, veh/h	0	r 51	TT 1313	1 29	0	TT 2125
Future Vol, veh/h	0	51 51	1313	29 29	0	2125
	0	0	1313	0	0	2125
Conflicting Peds, #/hr						
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	150	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	57	1459	32	0	2361
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	-	729	0	0	iviajoi z	
Stage 1		127	-	-		
Stage 2						
Critical Hdwy		6.94	-			
Critical Hdwy Stg 1						
	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	365	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	365	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annroach	WB		NB		SB	
Approach						
HCM Control Delay, s	16.7		0		0	
HCM LOS	С					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBT	
Capacity (veh/h)				365		
HCM Lane V/C Ratio				0.155		
HCM Control Delay (s)				16.7		
HCM Lane LOS				10.7 C		
HCM 95th %tile Q(veh)		-		0.5		
ncivi 95tii %tile Q(veii)		-	-	0.5	-	

			•			_	₽	1	†		-	¥	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<u></u>	7	ሻሻ	7	WEIN	1100	Ä	↑ ↑	7	ኝ	^	7
Traffic Volume (vph)	113	120	120	202	129	32	23	227	1900	166	96	1720	50
			120	202	129	32	23	227	1900	166	96	1720	50
future Volume (vph)	113	120											
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
ane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12	12
rade (%)		-2%			8%	_			0%			0%	
Storage Length (ft)	150		350	235		0		400		150	150		65
Storage Lanes	1		1	2		0		1		1	1		1
aper Length (ft)	100			200				100			100		
Satd. Flow (prot)	1787	1881	1599	3296	1735	0	0	1770	3539	1583	1770	3539	1583
It Permitted	0.445			0.950				0.062			0.054		
Satd. Flow (perm)	837	1881	1599	3296	1735	0	0	115	3539	1583	101	3539	1583
Right Turn on Red			Yes			Yes				Yes			Yes
Satd. Flow (RTOR)			132		7					129			83
ink Speed (mph)		45			45				55			55	
ink Distance (ft)		697			582				1856			1251	
ravel Time (s)		10.6			8.8				23.0			15.5	
Confl. Peds. (#/hr)													
Confl. Bikes (#/hr)													
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Frowth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
leavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)	U	U	U	U	U	U	U	U	U	U	U	U	U
Mid-Block Traffic (%)		0%			0%				0%			0%	
Shared Lane Traffic (%)		0%			0%				0%			U%	
` '	111	101	101	204	162	0	0	252	1919	168	97	1737	51
ane Group Flow (vph)	114	121	121			U							
urn Type	D.P+P	NA	pm+ov	Prot	NA		D.P+P	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	7	4	5!	3	8		5!	5	2	3	1	6	7
Permitted Phases	8		4				6	6		2	2		6
Detector Phase	7	4	5	3	8		5	5	2	3	1	6	7
Switch Phase									44.0				
/linimum Initial (s)	7.0	7.0	7.0	7.0	7.0		7.0	7.0	14.0	7.0	7.0	14.0	7.0
Minimum Split (s)	13.0	14.0	14.0	14.0	14.0		14.0	14.0	21.0	14.0	14.0	21.0	13.0
otal Split (s)	30.0	25.0	25.0	20.0	15.0		25.0	25.0	75.0	20.0	20.0	70.0	30.0
Fotal Split (%)	21.4%	17.9%	17.9%	14.3%	10.7%		17.9%	17.9%	53.6%	14.3%	14.3%	50.0%	21.4%
/ellow Time (s)	3.0	4.7	3.0	3.0	4.7		3.0	3.0	5.2	3.0	3.0	5.2	3.0
All-Red Time (s)	2.9	2.0	3.3	3.2	2.0		3.3	3.3	1.7	3.2	3.9	1.7	2.9
ost Time Adjust (s)	-0.9	-1.7	-1.3	-1.2	-1.7			-1.3	-1.9	-1.2	-1.9	-1.9	-0.9
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0			5.0	5.0	5.0	5.0	5.0	5.0
_ead/Lag	Lead	Lag	Lag	Lead	Lag		Lag	Lag	Lag	Lead	Lead	Lead	Lead
_ead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None		None	None	C-Max	None	None	C-Max	None
Act Effct Green (s)	35.0	21.0	41.0	14.0	21.7			85.0	73.7	92.6	85.0	65.0	78.3
Actuated g/C Ratio	0.25	0.15	0.29	0.10	0.16			0.61	0.53	0.66	0.61	0.46	0.56
//c Ratio	0.38	0.43	0.22	0.62	0.59			0.82	1.03	0.15	0.49	1.06	0.06
Control Delay	42.4	59.9	3.9	69.0	63.0			70.2	62.1	2.9	29.3	75.6	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0	0.0	0.0	0.0	0.0
otal Delay	42.4	59.9	3.9	69.0	63.0			70.2	62.1	2.9	29.3	75.6	0.4
.OS	D	Е	А	E	E			E	E	A	С	Е	Α
Approach Delay		35.3		_	66.3			_	58.7			71.2	
Approach LOS		D			E				50.7 E			7 1.2 E	
Queue Length 50th (ft)	80	102	0	92	133			172	~974	11	33	~910	0
Queue Length 95th (ft)	133	169	30	136	#243			#314	#1164	39	92	#1049	3
nternal Link Dist (ft)	133	617	30	130	502			#31 4	1776	37	72	1171	
Furn Bay Length (ft)	150	017	350	235	302			400	1770	150	150	11/1	65
Base Capacity (vph)	408	282	561	353	275			306	1862	1101	242	1643	1047
Starvation Cap Reductn	408	282	0	353	0			306	1862	0	0	1043	1047
	0	0	0	0	0			0	0	0	0	0	0
Spillback Cap Daduata	()	U	U					0	0	0	0	0	0
Spillback Cap Reductn		0	0	^							- ()	U	
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	0 0.28	0.43	0 0.22	0 0.58	0 0.59			0.82	1.03	0.15	0.40	1.06	0.05

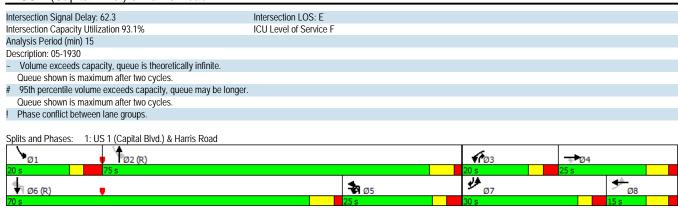
Area Type: Cycle Length: 140 Other

Actuated Cycle Length: 140
Offset: 25 (18%), Referenced to phase 2:NBSB and 6:NBSB, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.06



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Lane Group	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR
Lane Configurations	¥			ă	^	Đ	^	7
Traffic Volume (vph)	4	12	4	12	2362	13	2051	4
Future Volume (vph)	4	12	4	12	2362	13	2051	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12
Grade (%)	0%				0%		0%	
Storage Length (ft)	0	0		250		225		75
Storage Lanes	1	0		1		1		1
Taper Length (ft)	25			100		100		
Satd. Flow (prot)	1651	0	0	1770	3539	1770	3539	1583
Flt Permitted	0.988			0.950		0.950		
Satd. Flow (perm)	1651	0	0	1770	3539	1770	3539	1583
Link Speed (mph)	25				55		55	
Link Distance (ft)	716				691		210	
Travel Time (s)	19.5				8.6		2.6	
Confl. Peds. (#/hr)								
Confl. Bikes (#/hr)								
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0
Parking (#/hr)								
Mid-Block Traffic (%)	0%				0%		0%	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	17	0	0	17	2624	14	2279	4
Sign Control	Stop				Free		Free	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 75.3%
Analysis Period (min) 15

Intersection										
Int Delay, s/veh	2.6									
Movement	EBL	EBR	NBU	NBL	NBT	SBU	SBT	SBR		
Lane Configurations	¥			ă	^	Ð	^	7		_
Traffic Vol, veh/h	4	12	4	12	2362	13	2051	4		
Future Vol, veh/h	4	12	4	12	2362	13	2051	4		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	-	None	-	-	None	-	-	None		
Storage Length	0	-	-	250	-	225	-	75		
Veh in Median Storage, #	0	-	-	-	0	-	0	-		
Grade, %	0	-	-	-	0	-	0	-		
Peak Hour Factor	90	90	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2	2	2		
Mvmt Flow	4	13	4	13	2624	14	2279	4		
Major/Minor	Minor2		Major1			Major2				
Conflicting Flow All	3656	1139	1663	2279	0	1916	-	0		
Stage 1	2308	-	-	-	-	-	-	-		
Stage 2	1348	-	-	-	-	-	-	-		
Critical Hdwy	6.84	6.94	6.44	4.14	-	6.44	-	-		
Critical Hdwy Stg 1	5.84	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	5.84	-	-	-	-	-	-	-		
Follow-up Hdwy	3.52	3.32	2.52	2.22	-	2.52	-	-		
Pot Cap-1 Maneuver	~ 4	195	123	220	-	84	-	-		
Stage 1	61	-	-	-	-	-	-	-		
Stage 2	207	-	-	-	-	-	-	-		
Platoon blocked, %					-		-	-		
Mov Cap-1 Maneuver	~ 4	195	179	179	-	84	-	-		
Mov Cap-2 Maneuver	~ 4	-	-	-	-	-	-	-		
Stage 1	61	-	-	-	-	-	-	-		
Stage 2	207	-	-	-	-	-	-	-		
Approach	EB		NB			SB				
HCM Control Delay, s	\$ 646.9		0.2			0.4				
HCM LOS	F		0.2			0.1				
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBU	SBT	SBR			
		179	INDI	15	84	SDI	SDR			
Capacity (veh/h)		0.099	-	1.185	0.172		-			
HCM Captrol Doloy (c)		27.3	-		56.6	-	-			
HCM Control Delay (s) HCM Lane LOS		27.3 D	-	\$ 646.9 F	56.6 F		-			
HCM 95th %tile Q(veh)		0.3	-	2.8	0.6	-	-			
		0.3		2.0	0.0	_				
Notes										
~: Volume exceeds capacity	\$: Delay	exceeds	300s -	: Computa	tion Not D	efined	*: All majo	or volume	in platoon	

	۶	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	*	*	1	*	1
Traffic Volume (vph)	182	219	230	88	84	143
Future Volume (vph)	182	219	230	88	84	143
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	110			160	175	0
Storage Lanes	1			1	1	1
Taper Length (ft)	100				100	
Satd. Flow (prot)	1770	1863	1863	1583	1770	1583
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1770	1863	1863	1583	1770	1583
Link Speed (mph)		45	45		25	
Link Distance (ft)		582	1253		715	
Travel Time (s)		8.8	19.0		19.5	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	214	258	271	104	99	168
Sign Control		Free	Free		Stop	

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 36.8%
Analysis Period (min) 15

Intersection							
Int Delay, s/veh	6						
	EBL	EBT	WBT	WBR	SBL	SBR	
Movement							
Lane Configurations	100	210	220	7	\	142	
Traffic Vol, veh/h	182 182	219	230	88 88	84 84	143	
Future Vol, veh/h		219	230			143	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	110	-	-	160	175	0	
Veh in Median Storage, #	-	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	214	258	271	104	99	168	
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	271	0	iviajoi z	0	957	271	
Stage 1	-	-		-	271	- 2/1	
Stage 2				-	686		
Critical Hdwy	4.12	- :	-	-	6.42	6.22	
Critical Hdwy Stg 1	4.12	-		-	5.42	0.22	
Critical Hdwy Stg 2				_	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1292		-	-	286	768	
			-	-	775		
Stage 1	-	-	-	-		-	
Stage 2	-	-	-	-	500	-	
Platoon blocked, %	4000	-	-	-	000	7/0	
Mov Cap-1 Maneuver	1292	-	-	-	239	768	
Mov Cap-2 Maneuver	-	-	-	-	239	-	
Stage 1	-	-	-	-	775	-	
Stage 2	-	-	-	-	417	-	
Approach	EB		WB		SB		
HCM Control Delay, s	3.8		0		18.1		
HCM LOS	0.0		- 3		C		
200					J		
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)		1292		-	-	239	
HCM Lane V/C Ratio		0.166	-	-	-	0.413	
HCM Control Delay (s)		8.3	-	-	-	30.3	
HCM Lane LOS		Α	-	-	-	D	
HCM 95th %tile Q(veh)		0.6	-	-	-	1.9	

	-	•	•	—	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	?
Lane Configurations	f)			4	¥		
Traffic Volume (vph)	297	6	25	291	26	15	j
Future Volume (vph)	297	6	25	291	26	15	j
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900)
Lane Width (ft)	12	12	12	12	12	12)
Grade (%)	0%			0%	0%		
Storage Length (ft)		0	0		0	0)
Storage Lanes		0	0		1	0)
Taper Length (ft)			100		25		
Satd. Flow (prot)	1857	0	0	1855	1715	0)
Flt Permitted				0.996	0.969		
Satd. Flow (perm)	1857	0	0	1855	1715	0)
Link Speed (mph)	45			45	25		
Link Distance (ft)	1253			503	515		
Travel Time (s)	19.0			7.6	14.0		
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90)
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%)
Bus Blockages (#/hr)	0	0	0	0	0	0)
Parking (#/hr)							
Mid-Block Traffic (%)	0%			0%	0%		
Shared Lane Traffic (%)							
Lane Group Flow (vph)	337	0	0	351	46	0)
Sign Control	Free			Free	Stop		

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 46.0%
Analysis Period (min) 15

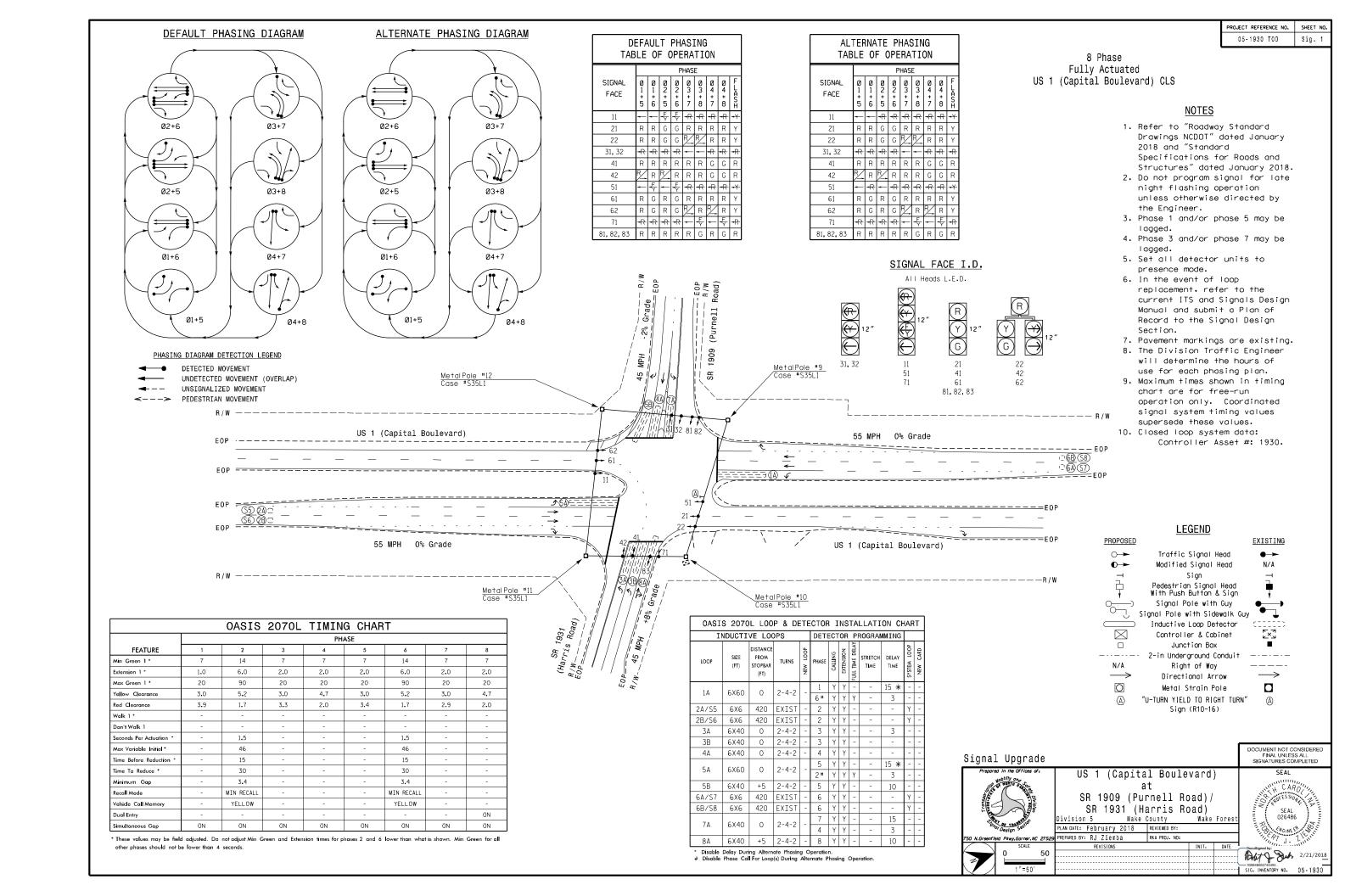
Intersection						
Int Delay, s/veh	1.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			र्स	¥	
Traffic Vol, veh/h	297	6	25	291	26	15
Future Vol, veh/h	297	6	25	291	26	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	330	7	28	323	29	17
WWITE F TOW	330	,	20	323	21	17
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	337	0	712	333
Stage 1	-	-	-	-	333	-
Stage 2	-	-	-	-	379	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1		-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1222	-	399	709
Stage 1					726	
Stage 2		_	_		692	_
Platoon blocked, %					0,2	
Mov Cap-1 Maneuver		_	1222	_	388	709
Mov Cap-1 Maneuver		-	1222	-	388	-
Stage 1			-	-	726	-
		-		-	673	-
Stage 2			-	-	0/3	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.6		13.6	
HCM LOS					В	
NA:		NDL 4	EDT	EDD	WDI	WOT
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		465	-	-	1222	-
HCM Lane V/C Ratio		0.098	-	-	0.023	-
HCM Control Delay (s)		13.6	-	-	8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.3	-	-	0.1	-

	•	4	†	1	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		#	^	7		^
Traffic Volume (vph)	0	34	2282	95	0	2065
Future Volume (vph)	0	34	2282	95	0	2065
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%		0%			0%
Storage Length (ft)	0	0		150	0	
Storage Lanes	0	1		1	0	
Taper Length (ft)	25				25	
Satd. Flow (prot)	0	1611	3539	1583	0	3539
Flt Permitted						
Satd. Flow (perm)	0	1611	3539	1583	0	3539
Link Speed (mph)	25		55			55
Link Distance (ft)	823		210			1856
Travel Time (s)	22.4		2.6			23.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	38	2536	106	0	2294
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 73.1%
Analysis Period (min) 15

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Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	7	† †	7	JDL	† †
Traffic Vol, veh/h	0	34	2282	95	0	2065
Future Vol, veh/h	0	34	2282	95	0	2065
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length		0		150		-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0		0			0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	38	2536	106	0	2294
WWITE	0	30	2330	100	U	2277
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	-	1268	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	160	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		160	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2			-	-	-	
, and the second						
Approach	WB		NB		SB	
HCM Control Delay, s	34.3		0		0	
HCM LOS	54.5 D		U		U	
TICIVI EOS	D					
		NOT	NDD	IIIDI 4	007	
Minor Lane/Major Mvmt		NBT		WBLn1	SBT	
Capacity (veh/h)		-	-	160	-	
HCM Lane V/C Ratio		-	-	0.236	-	
HCM Control Delay (s)		-	-	34.3	-	
HCM Lane LOS		-	-	D	-	
HCM 95th %tile Q(veh)		-	-	0.9	-	

Appendix I: Signal Plans & Timing Data



Field Notes - 11/9/2018

Signal ID: 05-1930

US 1 at Harris Road/Purnell Road

Phase: Extension 1: Yellow: Red: Dual Entry:	1 3 3.0 3.9	2 6 5.2 1.7	3 3 3.0 3.2	4 3 4.7 2.0 ON	5 3 3.0 3.3	6 6 5.2 1.7	7 3 3.0 2.9	8 3 4.7 2.0 ON
Plan 11 Time: Offset: Cycle: Splits:	6:30-9:30 AM 119 140 15	90	15	20	15	90	15	30
Plan 14 Time: Offset: Cycle: Splits:	2:35-8:00 PM 25 140 20	70	20	25	25	70	30	15